



January 2018

ZITHOLELE CONSULTING (PTY) LTD

Surface Water Impact Assessment and Baseline Report for Medupi Power Station - FGD Project

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REPORT

Report Number: 1415879-310165-2

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EXECUTIVE SUMMARY

Overview

Golder Associates Africa (Pty) Ltd was commissioned by Zitholele Consulting Services (Pty) Ltd (Zitholele) to prepare a surface water impact assessment for the Environmental Authorisation Process for the Medupi Power Station (Medupi) Flue Gas Desulphurisation (FGD) Retrofit Project for the following scope to support the Water Use Licence Application (WULA) required by the Department of Water and Sanitation (DWS):

- 1) Construction and operation of the FGD system within the Medupi Power Station Footprint (including the Zero Liquid Discharge Plant and temporary waste storage area);
- 2) Construction and operation of the railway yard and limestone and gypsum handling facilities between the Medupi Power Station plant and existing ADF; including two diesel storage facilities; and
- 3) Consideration of potential impacts from the disposal of ash and gypsum together on the existing ADF (that will necessitate a height change from 60m to 72m) for the amendment application of the existing Waste Management License.

The surface water impact assessment report will form one of the specialist investigations, and will be incorporated into the Integrated Environmental Assessment (EA1) and Water Use Licence Application for the proposed Medupi Power Station FGD retrofit project.

Methodology

The surface water impact assessment was carried out in three phases, namely:

- A desktop study to characterise the site, identify water sampling points and to conduct hydrological characterisation, catchment and water use description. The Mean Annual Precipitation (MAP), Mean Annual Runoff (MAR) and Mean Annual Evaporation (MAE) were determined from available data. Storm rainfall depths were obtained from the closest rainfall station for the 1:2, 1:5, 1:10, 1:20, 1:50, 1:100 and 1:200 year recurrence intervals, using the Design Rainfall Estimation Programme (Smithers & Schulze, 2002);
- A site visit to assess the site characteristics and collect surface water quality samples (where possible). The proposed water quality sampling points were dry, therefore, no water quality data was collected;
- A floodline determination exercise was carried out for the non-perennial tributary of the Sandloop River which is located to the west of the existing ADF and drains in a south-easterly direction;
- Report compilation including the following;
 - Water quality baseline status benchmarked against the South African Water Quality Guidelines for Industry (Category 1 Industrial Processes);
 - Potential impacts identification, rating pre- and post-mitigation for a list of anticipated activities;
 - Recommendation of mitigation measures to minimise or reduce impacts on the surface water quality and quantity; and
 - Develop a water quality and quantity monitoring programme indicating monitoring points, frequency of monitoring, database management and reporting.

Existing Environment

Medupi Power Station is located about 15km west of the town Lephalale in Limpopo Province. The power station is situated on 883 hectares that historically operated as a game and livestock farm (Bohlweki, 2005) and it has a design lifespan of 50 years. Baseline hydrology can be summarised as follows:



- The study area is located within the Limpopo Water Management Area (WMA) and within quaternary catchment A42J;
- Based on South African Weather Services (SAWS) weather station number 0717595_W and the DWS's weather station A4E003, the MAP and MAE for the study area were determined to be 416.09mm and 2 572mm respectively;
- Non-perennial streams, mainly the Sandloop River, drain the study area. The general drainage of the area is in an easterly direction towards the Mokolo River. These non-perennial streams in the area were found to be seasonal and only likely to flow after rainfall events;
- The study area has gentle slopes of 0.5% to 5% in general with relatively steeper slopes to the south of the study area;
- A visual inspection of soils in carried out during the site visit in November 2016. The soils were found to be sandy and well drained; and
- The Medupi catchment is characterised by natural woodland and game and cattle farming.

Water Quality

In order to establish baseline water quality for the study area prior to the construction of the FGD and the expansion of the existing ADF, a water quality monitoring programme was established by Golder in 2015. Baseline water quality can be summarised as follows:

- Due to lack of flow, no water samples have been collected at the 5 monitoring points at this stage;
- Water quality data obtained from the Wetland Assessment (Natural Scientific Services, 2015) has been utilised for water quality analysis; but
- Golder has put forward a recommendation for continuous monthly water quality monitoring at the 5 proposed locations.
- Samples should be taken monthly or when water is present at the proposed locations. During the dry season, each monitoring site should be visited every two to three months to see if there is water that can be sampled; and
- The parameters to be analysed should include:
 - pH, Total Dissolved Solids, Electrical Conductivity, Alkalinity, Potassium, Calcium, Sodium, Chloride, Fluoride, Sulphate, Nitrate, Ammonium, Total Hardness, Metals: Arsenic, Beryllium, Cadmium, Barium, Chromium, Copper, Lead, Mercury, Molybdenum, Nickel, Selenium, Uranium, Vanadium and Zinc using ICP-MS), Orthophosphate, Total Suspended Solids, Oil and Grease

Water Management System

During the site visit conducted Golder, a water management system was identified. The water management system is aimed at mitigating the impact of the existing Medupi project on downstream water quality. However,

The existing water management system includes:

- A dirty water management system to ensure that polluted water the power station and its associated infrastructure, including the existing ADF, as well as sediment-laden runoff from disturbed areas is separated from clean area runoff and that it is collected in Pollution Control Dams (PCD); and
- A clean water management system to divert water undisturbed by the power station's operations around the disturbed project footprint.



Detailed storm water management design reports are available for the railway and ash dump. It is important that these are implemented to ensure adequate storm water control.

Water Balance

A numerical water balance model was developed for the existing operations at Medupi Power Station in order to assess the effectiveness of the power station water management system. However, a copy of this study has not been obtained at this stage and therefore no further reference can be made or conclusions drawn from it.

It is nonetheless recommended that a revision of this water balance study be carried out to include the FGD retrofit project as well as the proposed expansion of the existing ADF.

Flooding

The footprint of the proposed Ash Disposal Facility is 925.86 ha (9.26 km²). The following summary can be made from the floodline study:

- The 1:100 year floodline encroaches on the ADF footprint;
- The south-western portion of the proposed ADF footprint, where a pollution control dam is located will be mostly affected by the 1 in 100 flood;
- The ADF project disturbance boundary is located within the Probable Maximum Flood (PMF); therefore
- To avoid flooding and contamination of the downstream environment through the transportation of pollutants from the ADF, the storm water management must be designed in such a way to mitigate pollution from the PCD to the river; and
- Water quality monitoring in the PCD located on the south west corner must be undertaken monthly or in accordance with the relevant water use authorisation, so that any potential impacts can be detected and mitigation planned.

Loss of Catchment Flows

- The existing Medupi site and ADF site have a combined area of approximately 1,874 ha (18.7 km²) which equates to 1.03% of quaternary catchment A42J with a catchment area of 1,812 km² (WRC, 2012);
- The Sandloop River tributary has an estimated catchment area of 4,467 ha (44.7 km²). The reduction in catchment area from the Medupi site and ADF site of approximately 1,874 ha (18.7 km²) equates to a 49.95% decrease in catchment area; and
- It is therefore anticipated that during the operational phase of the ADF, there will be a reduction in the total runoff reporting to the Sandloop River tributary, however limited reduction to the Mokolo system.

Mitigation and Management Measures

The following conclusions were drawn and recommendations made from the Medupi surface water impact assessment study:

- Natural on land surface water drainages are absent in the existing footprint of Medupi Power Station and will therefore not be impacted by the proposed Flue Gas Desulphurisation (FGD) Retrofit project.
- The 100-year floodline of the Sandloop River in the area of the ADF encroaches on the ADF footprint in the south western corner and this may have a detrimental effect in the event of a major flood event. Should the ADF operate within the 1:100 year floodline, the risk of pollutant transportation towards downstream water users during a flood event will be elevated. This will include flooding of the disposal facility and entrainment of waste materials and sediments downstream, making the management of the facility during significant storm events very difficult.



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- If sound engineering flood control and prevention measures are not put in place, the contents of the ADF are likely to be washed away into the receiving environment in the event of a 1:100 flood. Statistically, the 1:100 year flood event refers to the mathematical probability of this flood magnitude occurring once over a 100-year period. However, in reality this flood magnitude may occur more than once in 100 years. With this in mind, the 20-year lifespan of the ash disposal facility should not be directly compared to the 1:100 flood event. ADF design and flood mitigation measures should be based on the 1:100 year flood event.
- Storm water that is generated within the Medupi Power Station, including the ADF, as a result of rainfall is a route by which pollutants may be mobilised and transported into the receiving downstream environment. The National Water Act (NWA) prohibits the discharge of any effluent (including contaminated storm water) into any water resources.
- To prevent possible pollution of the receiving surface water environment, dirty water containment structures should be designed, constructed, maintained and operated such that they do not spill over more than once in 50 years. A minimum freeboard of 0.8 m above full supply level (FSL) must also be maintained as per GN704 requirements (flow-based hydraulic sizing requirements). Water accumulated in the containment facility during the wet season should be used as a priority in the process water circuit to ensure that the capacity requirements are not compromised during periods of heavy and/or extended rainfall.
- It is recommended that an update to both the storm water management plan (SWMP) and the existing water balance be undertaken such that it caters for the proposed FGD and ADF infrastructure as well as be designed and operated in line with the DWS's GN704.
- During construction and times of major disturbances to land cover, it is recommended that sound engineering measures are put in place to protect the receiving surface water environment. It is also recommended that, where possible, construction and land cover disturbance is carried out during the dry season to avoid the washing away of materials by surface runoff (post-construction sediment and erosion control).
- If possible, it is recommended that a detention (dry) pond be constructed at or near the discharge point of the clean water drainage system before it enters the environment, or the clean water system be designed in such a manner to allow for longer residence times. This pond would be constructed for the purpose of flood control as well as storm water runoff treatment. This pond will function to settle suspended sediments and other solids typically present in storm water runoff. In the event of a major storm, the detention pond will slow down water flow and hold it for a short period of time before releasing it to the environment. Should the second option of designing the clean water system for a longer residence time be considered, then additional maintenance for periodic removal of collected sediment would be required.
- It is strongly recommended that the proposed water quality monitoring programme be strictly followed and sustained so that chemical constituent levels can be monitored and analysed over time. Pollution of surrounding surface water features should be avoided at all costs during the lifespan of the Medupi Power Station project. In the unfortunate occurrence of surface water resources pollution, swift and effective corrective measures should be implemented and the relevant authorities notified without delay.
- With respect to the transportation of sludge and salts from Medupi to a hazardous waste disposal site, it is recommended that a route selection study be carried out to determine the least potential water surface impacts, considering other factors such as the traffic impact assessment. From a surface water perspective, a route via a national road (highway) would be most appropriate as the likelihood of accidents and spillages due to poor road conditions will be minimised.

The impact assessment showed that most impacts were low after mitigation. If the impacts are properly mitigated and Best Management Practices followed at all times, the identified potential impacts can be reduced to negligible.



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APPENDICES

APPENDIX A

Document Limitations



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APPENDIX B

General layout of the existing ADF and storm water management philosophy



1.0 INTRODUCTION

Golder associates Africa (Pty) Ltd (Golder) was appointed by Zitholele Consulting Services (Pty) Ltd (Zitholele) to assess the potential surface water impacts at Medupi Power Station (Medupi) in relation to the following scope:

- 1) Construction and operation of a rail yard/ siding to transport Limestone from a source defined point via the existing rail network to the Medupi Power Station and proposed rail yard / siding. The rail yard infrastructure will include storage of fuel (diesel) in above ground tanks and 15m deep excavation for tippler building infrastructure;
- 2) Construction and operation of a limestone storage area, preparation area, handling and transport via truck and conveyor to the FGD system located near the generation units of the Medupi Power Station;
- 3) The construction and operation of the wet FGD system that will reduce the SO₂ content in the flue gas emitted;
- 4) Construction and operation of associated infrastructure required for operation of the FGD system and required services to ensure optimal functioning of the wet FGD system. The associated FGD infrastructure include a facility for storage of fuel (diesel), installation of storm water infrastructure and conservancy tanks for sewage;
- 5) The handling, treatment and conveyance of gypsum and effluent from the gypsum dewatering plant. Disposal of gypsum on the existing ADF is not included in the current EIA application and will be addressed in the ADF Waste Management License (WML) amendment application.
- 6) Pipeline for the transportation of wastewater from the gypsum dewatering plant and its treatment at the WWTP (Zero Liquid Discharge (ZLD) plant) that will be located close to the FGD infrastructure within the Medupi Power Station;
- 7) Construction and operation of the ZLD plant;
- 8) Management, handling, transport and storage of salts and sludge generated through the waste water treatment process at a temporary waste storage facility. In terms of the EIA process impacts related to the management of salts and sludge will be considered in the EIR. However, licencing of the storage activity and requirements relating to the waste storage facility will be assessed in the WML registration application process.
- 9) The transportation of salts and sludge via trucks from the temporary waste storage facility to a final Waste Disposal Facility to be contracted by Eskom for the first 5 years of operation of the FGD system. Long term disposal of salts and sludge will be addressed though a separate independent EIA process to be commissioned by Eskom in future.
- 10) Disposal of gypsum together with ash on the existing licenced ash disposal facility (ADF), with resulting increase in height of the ADF from 60m to 72m.

Medupi currently has a Water Use Licence (WUL) for its existing industrial footprint, including the existing ADF which will be constructed in three phases. Currently the power station is ashing on the four years ashing cell. This study focuses on the environmental authorisation process and Water Use Licence Application (WULA) for the Medupi Power Station Flue Gas Desulphurisation (FGD) as well as the proposed expansion of the existing ADF from 4 years to 20 years operational lifespan. The existing Medupi Power Station has been designed and constructed to be FGD retrofit ready, therefore the potential surface water impacts of the FGD process will occur on an already impacted footprint.

1.1 Project Description

Medupi Power Station (Medupi) is a coal-fired power station that forms part of the Eskom New Build Programme. This project focuses on the environmental authorisation process for the Medupi Power Station Flue Gas Desulphurisation (FGD) Retrofit. As part of the Environmental Impact Assessment (EIA) and Water Use Licence Application (WULA), it is required that a Surface Water Impact Assessment (SWIA) be



conducted. This report provides the surface water impact assessment as well as baseline water quality and quantity for the existing licensed ADF and Medupi Power Station footprint in general. This surface water report that will form part of supporting documentation for the Environmental Impact Assessment (EIA), is based on desktop studies of available literature and aerial imagery.

From initial site investigations, it would seem that the key watercourse for floodline delineation and impact assessment would be the Sandloop River which generally drains the entire existing Medupi Power Station footprint.

The surface water impacts that could arise through transportation of sludge and salts from the Medupi Power Station to an appropriately licensed existing hazardous waste disposal facility is also a subject of discussion in this report. A qualitative specialist opinion is given in this regard.

1.2 Scope of Work

The detailed scope of work for the surface water impact assessment component included:

- Undertake any further site investigations/modelling as necessary;
- Identify watercourses within 500 m of the power station footprint and disposal facility footprint for flood line delineation and assessment;
- Assess the potential surface water impacts generated by the construction, operation, and decommissioning of the existing waste disposal facility for ash and gypsum disposal;
- Provide a specialist opinion on the significance of the surface water impacts for the proposed trucking of sludge and salts to an existing licensed Hazardous Waste Disposal Facility, outside of the Medupi Power Station study area;
- Attend one specialist integration meeting in Midrand, Waterfall City, to discuss the ratings and integrate the assessments for purposes of the EIA;
- Compile an Impact Assessment Report, with one round of review from Zitholele and Eskom;
- Attend the EIA Phase public meeting in Lephalale, Limpopo Province;
- The following tasks will be carried out in order to achieve the scope of work:
 - Compile a map showing the catchment areas, site infrastructure and the major surface water drainage lines;
 - Collect the available daily rainfall data from client records, South African Weather Services (SAWS) or Department of Water and Sanitation (DWS) and check for integrity. The rainfall data will be patched to produce a daily rainfall record for use in surface water modelling;
 - Rainfall statistics such as monthly averages, number of rain days per month, distribution of annual totals and the 2, 5, 10, 20, 50, 100 and 200 year recurrence interval 24 hour storm depths will be determined;
 - Collect and review the available climate data to produce monthly potential evaporation and temperature statistics based on regional and local climatic data;
 - Conduct a two day site visit entailing site familiarization and measurement of all river crossings including bridges, culverts, pedestrian pathways, railway crossings, pipelines, etc.;
 - Map and describe the surface water resources in the study area;
 - Propose and implement a water quality programme for the drainages that could be impacted on by the existing operations and proposed sites at Medupi;



- Calculate the 1:50 and 1:100 year peak using the Rational Method and also determine the 1:50 and 1:100 year floodline based on the current development levels and taking into account all current infrastructure using HEC-RAS,
- Use the IA Rating System as provided by Zitholele Consulting to quantify the surface water impact; and
- Compile an Impact Assessment Report which identifies potential impacts on surface water and provides significance ratings for the impacts, as well as proposed mitigation actions.

1.3 Study Area

Medupi Power Station is situated in the Matlabas catchment which is a predominantly flat area of the Limpopo Water Management Area (WMA). Medupi is approximately 19 km west of the town of Lephalale and approximately 42 km south of the Limpopo River. The catchment is still largely undeveloped with limited water resources and water uses. The Medupi site is situated in the Steenbokpan area which lies in the A42J quaternary catchment. The sites investigated are located west of Medupi Power Station. Figure 1 shows the study area in relation to the surface water resources.

This study area is located to the south of the Lephalale coalfield and numerous mining developments are foreseen, predominantly to the north of the Eenzaamheid Fault line. The area is a semi-arid region which has economic activity centred on livestock farming, irrigation and industrial development including coal mining and power generation. The Matlabas catchment is a dry catchment with non-perennial flow and therefore limited sustainable yield from surface water (Department of Water Affairs, 2013). Figure 1 shows the locality map along with the climate stations in the area.



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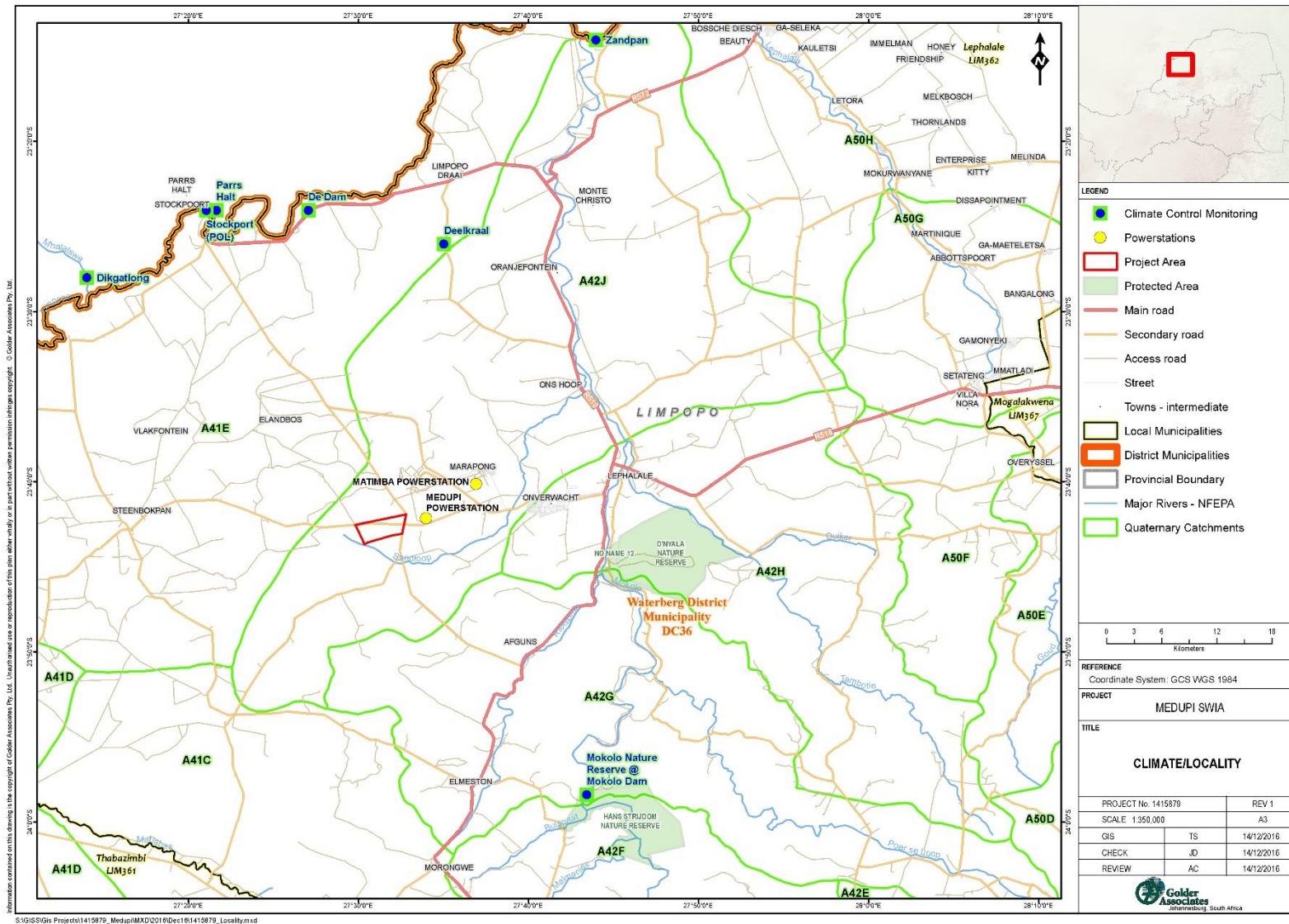


Figure 1: Locality and climate map for the Medupi area



1.4 Project Team

The Golder surface water team comprised of the following members:

Table 1: Surface water project team

| Name | Project role | Qualifications | Years of experience |
|----------------|---|-------------------------------------|---------------------|
| Lee Boyd | Project manager / Water Resource Scientist | MSc, SFWISA (Pr. Sci. Nat) | 25 years |
| Zinhle Sithole | Junior Hydrologist | BSc (Hons) Hydrology | 3 years |
| Johan Jordaan | Project Reviewer | BEng (Civil Engineering) (Pr. Eng.) | 20 years |
| Trevor Coleman | Technical Advisor / Senior Water Resources Engineer | MSc Engineering (Pr. Eng.) | 35 years |

1.5 Related Studies

The studies which are to be read in conjunction with this assessment include the following:

- Zitholele Technical Memorandum: Medupi PS FGD Retrofit Project, Project scope and description – December 2017;
- Integrated Environmental Authorisation Process for the Medupi Power Station Flue Gas desulphurisation (FGD) Retrofit Project, Final Scoping Report (Zitholele, 2015);
- A Wetland Assessment for the Ash Disposal Facility at Medupi Power Station – Lephalale, Limpopo (Natural Scientific Services, 2016);
- Knight Piésold, Conceptual Design of Stormwater Management, Sewage Infrastructure and Access Roads between Boiler Edge Slab and Road No.3 (Ring Road West) and Design of the New Gypsum Offtake Infrastructure Slab, associated Drainage, and Access Roads, October 2017.

1.6 Report Structure

This report comprises ten sections:

- Section 1 is the introduction and background to the project;
- Section 2 provides an overview of the regulatory framework related to the project;
- Section 3 describes the existing environment with respect to surface water resources;
- Section 4 outlines the existing surface water management system at Medupi Power Station;
- Section 5 provides a high level overview of the existing site water balance;
- Section 6 presents the floodline determination study for the Sandloop River tributary;
- Section 7 describes the potential impacts of the FGD retrofit project and ADF on surface water resources;
- Section 8 provides a specialist opinion on the transportation of sludge and salts from Medupi to an off-site licensed hazardous waste disposal facility;
- Section 9 outlines the proposed mitigation and management measures of the project; and
- Section 10 presents a summary of the conclusions and recommendations of the surface water impact assessment.



2.0 REGULATORY FRAMEWORK

2.1 Regulatory Documents

This part of the document is intended to detail environmental legislation that may have bearing on the existing Medupi Power Station project as well as the proposed expansion of the Ash Disposal Facility. The following national legislation, plans, policies and regulations are relevant to this project in terms of surface water management.

2.1.1 Constitution of the Republic of South Africa, 1996 (Act No. 108 of 1996)

The Constitution of the Republic of South Africa, 1996 (hereafter referred to as "the Constitution") is the Supreme Law in South Africa. The Bill of Rights is included in Chapter 2 of the Constitution. The Environmental Right as set out in Section 24 of the Constitution and states that – Everyone has the right –

- to an environment that is not harmful to their health or well-being; and
- To have the environment protected, for the benefit of present and future generations, through reasonable legislative and other measures that –
 - i) Prevent pollution and ecological degradation;
 - ii) Promote conservation; and
 - iii) Secure ecologically sustainable development and use of natural resources, while
 - iv) Promoting justifiable economic and social development.

The National Environmental Management Act, 1998 (Act No. 107 of 1998) is the primary statute which gives effect to Section 24 of the Constitution. The Environmental Right contained in Section 24 of the Constitution also places responsibility on the Environmental Assessment Practitioner (EAP), the Applicant and the Competent Authority to ensure that this right is not infringed upon. The Sector Guidelines for Environmental Impact Assessment (2010) (Government Notice 654) describes a number of responsibilities which are placed on the EAP, Applicant and Competent Authority to ensure conformance with the statutory Environmental Right.

2.1.2 National Water Act, 1998 (Act No. 36 of 1996)

The specialist surface water assessment complies with South African legislation for environmental authorisations, most specifically the National Water Act (NWA), 1998 (Act No. 36 of 1998). The activities associated with the proposed Medupi Power Station FGD retrofit project and Ash Disposal Facility trigger some of the Water Uses that are defined in Section 21 of the National Water Act, 1998 (Act No. 36 of 1998) (NWA). Accordingly these Water Uses may not be undertaken without being granted a Water Use License from the DWS. In accordance with Sections 40 and 41 of the NWA (1998), a Water Use License Application Process will be carried out. The resultant documents from the WULA process will include completed WULA Forms as well as a Technical Report. These documents will be submitted to DWS for review and decision making. Although a joint PPP is followed for the WULA within the EIA Phase, these two EA processes constitute separate applications and submissions are made to the respective Competent Authorities.

2.1.3 National Environmental Management Act, 1998 (Act No. 107 of 1998)

The Environmental Management can be defined as the management of human interaction with the environment. Fuggle and Rabie (Strydom & King; 2009) defines Environmental Management as the regulation of the effects of peoples' activities, products and services on the environment. Although South Africa has a comprehensive array of environmental legislation and policies in place, these must be aligned with the provisions of the NEMA (1998), in particular the National Environmental Management Principles stipulated in Chapter 1 of the NEMA (1998). The Environmental Management Principles are centred around providing explicit guidance for co-operative and environmental governance on all matters relating to decision-making which will affect the environment, institutions that will promote co-operative governance and procedures for co-ordinating environmental functions exercised by organs of state, and to provide for matters connected therewith.



2.2 Water Use Licence

The National Water Act (Act 36 of 1998) (NWA) identifies 11 consumptive and non-consumptive water uses which must be authorised under a tiered authorisation system. This authorisation system includes scheduled uses, general authorisations and water use licences. It allows for the "Reserve"¹ and provides for public consultation processes in the establishment of strategies and decision-making and guarantees the right to appeal against such decisions.

Sections 40 and 42 of the NWA provides for the responsible authority to request an assessment of the likely effect of the proposed license on the resource quality, and that such assessment is subject to the EIA regulations as promulgated under Section 26 of the Environmental Conservation Act, 1989 (Act 73 of 1989) (ECA).

In terms of Section 21 of the NWA which relates to the consumption of water, as well as activities which may affect water quality and the condition of the resource itself, the following water uses need to be authorised:

- Section 21(a) – Taking water from a water resource;
- Section 21(b) – Storing water;
- Section 21(c) – Impeding or diverting the flow of water in a watercourse;
- Section 21(d) – Engaging in a stream flow reduction activity;
- Section 21(e) – Engaging in a controlled activity;
- Section 21(f) – Discharging waste or water containing waste into a water resource through a pipe, canal, sewer or other conduit;
- Section 21(g) – Disposing of waste in a manner which may detrimentally impact on a water resource;
- Section 21(h) – Disposing in any manner of water which contains waste from, or which has been heated in any industrial or power generation process;
- Section 21(i) – Altering the bed, banks, course or characteristics of a watercourse;
- Section 21(j) – Removing, discharging or disposing of water found underground if it is necessary for the efficient continuation of an activity or for the safety of people; and
- Section 21(k) – Using water for recreational purposes.

Medupi has an existing IWUL. The water uses related to this project will be identified and an application made to the DWS. The surface impact assessment will also consider these aspects.

2.3 Water Resources Classification

The classification of significant water resources in the Crocodile (West), Marico, Matlabas and Mokolo catchments in accordance with the Water Resource Classification System (WRCS) was undertaken in 2011/2012 and finalised in 2013 (Department of Water Affairs, 2013). Classification of water resources aims to ensure that a balance is reached between the need to protect and sustain water resources on the one hand and the need to develop and use them on the other. The WRCS places the following principles at the forefront of implementation:

¹ "Reserve" means the quantity and quality of water required -
(a) to satisfy basic human needs
(b) to protect aquatic ecosystems in order to secure ecologically sustainable development and use of the relevant water resource;



- Maximising economic returns from the use of water resources;
- Allocating and benefits of utilising the water resources fairly; and
- Promoting the sustainable use of water resources to meet social and economic goals without detrimentally impacting on the ecological integrity of the water resource.

Each quaternary catchment is classified as a Class I, II or III, defined as:

- Class I - Minimally used: Water resource is one which is minimally used and the overall condition of that water resource is minimally altered from its pre-development condition;
- Class II - Moderately used: Water resource is one which is moderately used and the overall condition of that water resource is moderately altered from its pre-development condition; and
- Class III - Heavily used: Water resource is one which is heavily used and the overall condition of that water resource is significantly altered from its pre-development condition.

The recommended Class for quaternary catchment A42J is a Class II (Department of Water Affairs, 2013). In this respect mitigation implemented must be such that it will protect the water resources so that an ecological category of B/C is maintained. Ecological category refers to the assigned ecological condition by the Minister to a water resource that reflects the ecological condition of that water resource in terms of the deviation of its biophysical components from a pre-development condition. These ecological categories are in the order of A, B, C, and D with intermediate A/B, B/C, and C/D, where A is a well maintained ecological system and D is a poorly maintained system.

2.4 Best Management Practices

The Department of Water and Sanitation's (DWS) Best Practice guidelines (Department of Water Affairs and Forestry, 2006) and GN 704 of the National Water Act (National Water Act, 1999) will be used as a guiding principle. This section is extracted from the DWS's Guideline Document for the Implementation of Regulations on Use of Water for Mining and Related Activities Aimed at the Protection of Water Resources, Operation Guideline No. M6.1 of May 2000, Second Edition.

The Minister of Water and Sanitation (then known as Department of Water Affairs and Forestry, DWAF) promulgated regulations in respect of use of water for mining and related activities aimed at the protection of water resources on June 1999. These regulations are aimed at both at the mining industry (including industries with related activities, as defined) and the DWS who has to enforce the regulations. The final regulations were published in Government Notice No. 704 on 4 June 1999 (*Government Gazette* No. 20119) and approved by the National Assembly on 14 October 1999. The Minister of Water and Sanitation is responsible for the protection, conservation, management and control of water resources of South Africa on a sustainable basis. The requirements prescribed in terms of the regulations must be seen as minimum requirements to fulfil this goal.

During the development of the regulations a decision was made that industrial activities will not be included in the definition of "activity". However the differentiation between a mining and an industrial activity is not always that clear. When any doubt exists whether a specific activity directly or indirectly related to mining should comply with GN704 or not, the issue should be evaluated on a site-specific basis and a decision made on that basis. An example of the above differentiation is made below:

Eskom: Coal-fired power stations

The phrase "...whether situated at the mine or not..." allows for the following sections of the definition to be applicable to power station activities:

- Mineral storage yards, transport facilities and loading zones; and
- Storing, stockpiling, accumulating, dumping, disposing or transportation of residue.



A decision was made by DWS that coal-fired power stations are not included in the definition of “activity” as coal-fired power stations are regarded an industrial activity, and not a mining activity. Coal-fired power stations and its directly related activities are therefore excluded from these regulations. It is however important to note that, should a power station, for instance, make use of the workings of any underground or opencast mine excavation for the disposal of any residue defined in the regulations, the specific activity is considered a *related activity* and is thus not exempted from these regulations.

However, whenever making this differentiation between mining and industrial activities, the following must be kept in mind and the industrial activities excluded from the definition of “activity” must be advised accordingly:

- DWS is in the process of developing similar regulations on the use of water for industrial activities. These regulations will address the same concerns as that of GN704, and will most likely have similar requirements. It is therefore proposed that the industrial activities, especially new activities, address and manage their water-related issues according to these regulations;
- Section 19 of the National Water Act stipulates that all reasonable steps must be taken to prevent pollution from occurring, continuing or recurring from any activity or process which causes, has caused, or is likely to cause pollution of a water resource. The industrial activities excluded from the definition of “activity” are therefore not exempted from preventing or rectifying any pollution caused by their activities.

Stormwater Management Plan (SWMP)

The SWMP described in this section is developed to meet the requirements of GN704 of the National Water Act by;

- Diverting all clean water and, prevent any further runoff from entering mining or industrial areas;
- Directing any unpolluted water to a clean water system away from possible contamination;
- Design, construct, maintain and operate any clean water system at the mine or related activity so that it is not likely to spill into any dirty water system more than once in 50 years;
- Collect the water arising within any dirty area, including water seeping from mining operations, outcrops or any other activity, into a dirty water system.
- Design, construct, maintain and operate any dirty water system at the mine or related activity so that it is not likely to spill into any clean water system more than once in 50 years; and
- Design, construct, maintain and operate any dam or tailings dam that forms part of a dirty water system to have a minimum freeboard of 0.8 metres above full supply level.

One of the most important best management practice principles relating to water management is the separation of unpolluted (clean) and polluted (dirty) water and in order to achieve this effectively, the person in control of a mining or related activity should develop and implement a storm water management plan for their premises. GN704 was published to provide regulations on the use of water for mining and related activities aimed at the protection of water resources. There are important definitions in the regulation, which require understanding, and these are discussed below.

- **Clean water system:** This includes any dam, other form of impoundment, canal, works pipeline and any other structure or facility constructed for the retention or conveyance of unpolluted water.
- **Dam:** This includes any settling dam, slurry dam, evaporation dam, catchment or barrier dam and any other form of impoundment used for the storage of unpolluted water or water containing waste (i.e. polluted water).
- **Dirty area:** This refers to any area at a mine which causes, has caused or is likely to cause pollution of a water resource (i.e. polluted water).



- **Dirty water system:** This includes any dam, other form of impoundment, canal, works, pipeline, residue deposit and any other structure or facility constructed for the retention or conveyance of water containing waste.

The four main principle conditions of GN704 applicable to the Medupi Power Station project are:

- *Condition 4* which defines the area in which mine workings or associated structures may be located with reference to a watercourse and associated flooding. The 50 year floodline and 100 year floodline are used for defining suitable locations for mine workings and associated structures respectively. Where the floodline is less than 100 meters away from the watercourse, then a minimum watercourse buffer distance of 100 meters is required for both the workings and associated structures.
- *Condition 5* which indicates that no residue or substance which causes or is likely to cause pollution of a water resource may be used in the construction of any dams, impoundments or embankments or any other infrastructure.
- *Condition 6* which describes the capacity requirements of clean and dirty water systems. Clean and dirty water systems must be kept separate and must be designed, constructed, maintained and operated such that these systems do not spill over into each other more than once in 50 years.
- *Condition 7* which describes the measures that must be taken to protect water resources. All dirty water or substances which cause or are likely to cause pollution of a water resource either through natural flow or by seepage are to be mitigated.

3.0 EXISTING SURFACE WATER ENVIRONMENT

3.1 Regional Drainage Network

As shown in Figure 1, the study area is located within the A42J Quaternary catchment to the south of the Lephalale coalfield where numerous mining developments are foreseen predominantly to the north of the Eenzaamheid Fault line. There are no perennial streams originating within the area itself. The closest perennial river is the Mokolo into which the non-perennial Sandloop River drains. The Mokolo flows through A42J to the Limpopo River.

Medupi is situated in the Mokolo catchment, with the non-perennial Sandloop River flowing around the site in an easterly to north easterly direction to confluence with the Mokolo River approximately 16 kilometres downstream of the town of Lephalale.

This is a predominantly flat area of the Limpopo Water Management Area (WMA). Medupi is approximately 19 km west of the town of Lephalale and the Mokolo River and approximately 42 km south of the Limpopo River. Except for those areas where mining and power generation has commenced the catchment is still largely natural with limited cultivated areas. The water resources are also limited. Game farming is a common land use in the area. The town of Lephalale has seen considerable growth in the past decade.

3.1.1 Water Resource Classification and Resource Quality Objectives

The classification of significant water resources in the Crocodile (West), Marico, Matlabas and Mokolo catchments in accordance with the Water Resource Classification System (WRCS) was undertaken in 2011/2012 and finalised in 2013 (Department of Water Affairs, 2013). Classification of water resources aims to ensure that a balance is reached between the need to protect and sustain water resources on the one hand and the need to develop and use them on the other. The WRCS places the following principles at the forefront of implementation:

- Maximising economic returns from the use of water resources;
- Allocating and benefits of utilising the water resources fairly; and
- Promoting the sustainable use of water resources to meet social and economic goals without detrimentally impacting on the ecological integrity of the water resource.



Each quaternary catchment is classified as a Class I, II or III, defined as:

- Class I - Minimally used: Water resource is one which is minimally used and the overall condition of that water resource is minimally altered from its pre-development condition;
- Class II - Moderately used: Water resource is one which is moderately used and the overall condition of that water resource is moderately altered from its pre-development condition; and
- Class III - Heavily used: Water resource is one which is heavily used and the overall condition of that water resource is significantly altered from its pre-development condition.

The recommended Class for quaternary catchment A42J is a Class II (Department of Water Affairs, 2013). In this respect mitigation implemented must be such that it will protect the water resources so that an ecological category of B/C is maintained. Ecological category refers to the assigned ecological condition by the Minister to a water resource that reflects the ecological condition of that water resource in terms of the deviation of its biophysical components from a pre-development condition. These ecological categories are in the order of A, B, C, and D with intermediate A/B, B/C, and C/D, where A is a well maintained ecological system and D is a poorly maintained system.

The determination of Resource Quality Objectives (RQO) for the area was undertaken in 2016/ 2017 and will be gazetted during the first quarter of 2018 (DWS, 2017, Report number: DM/WMA01/00/CON/RQO/0516). The proposed RQOs and numerical limits are set out in Table 2.



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Table 2: RQOs and numerical limits for quaternary catchment A42J

| Component | Sub-component | RQO | Indicator | Numerical Limit | Context/Rationale for RQO/numerical limit |
|-----------|------------------|--|---|--|--|
| Quality | Nutrients | Instream concentration of nutrients must be maintained to sustain aquatic ecosystem health and ensure the prescribed ecological category is met. | Orthophosphate (PO ₄ ⁻) as Phosphorus | ≤ 0.05 milligrams/litre (mg/l) (50 th percentile) | Present ecological state maintained. Require baseline data. |
| | | | Nitrate (NO ₃ ⁻) & Nitrite (NO ₂ ⁻) as Nitrogen | ≤ 0.1 milligrams/litre (50 th percentile) | Present ecological state maintained. Require baseline data. |
| | Salts | Instream concentration of salinity must be maintained to protect present ecological state and the aquatic ecosystem health. | Electrical Conductivity | ≤ 55 milliSiemens/metre (mS/m)(95 th percentile) | Maintain present water quality. |
| | System Variables | pH range must be maintained within limits specified to support the aquatic ecosystem and water user requirements. A baseline assessment to determine the present state instream turbidity is required. Limits must be defined to control the impacts of slate mining on the resource. | pH range | 6.5 (5 th percentile) and 8.5 (95 th percentile) | Aquatic ecosystem as the driver. Present ate |
| | | | Turbidity | A 10% variation from background concentration is allowed. Limits must be determined. | No baseline data available. Monitoring required to determine present state. |
| | Toxics | The concentrations of toxicants must pose no risk to aquatic organisms and to human health. | Atrazine | ≤0.078 milligrams/litre (mg/l) | Human health is the driver. Aquatic ecosystem is the driver. Ecological specification. Ecological Reserve manual (2008). No monitoring data. |
| | | | Imidacloprid | ≤ 0.000038 milligrams/litre (mg/l) | Human health considerations. Environment Protection Authority of New Zealand – Environmental Exposure Limit |
| | | | Aluminium (Al) | ≤ 0.062 milligrams/litre (mg/l)(95 th percentile) | Strictest of Ecological specifications for all metals except manganese. Manganese – domestic |
| | | | Manganese (Mn) | ≤ 0.15 milligrams/litre (mg/l) (95 th percentile) | |
| | Iron (Fe) | ≤ 0.1 milligrams/litre (mg/l) | | | |



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| | | | | | |
|---------|------------------|--|---|--|--|
| | | | | (95th percentile) | user requirements. Ecological Reserve manual (2008), South African Water Quality Guidelines (1996) |
| | | | Lead (Pb) hard | ≤ 0.0057 milligrams/litre (mg/l) (95th percentile) | |
| | | | Copper (Cu) hard | ≤ 0.0048 milligrams/litre (mg/l) (95th percentile) | |
| | | | Nickel (Ni) | ≤ 0.07 milligrams/litre (mg/l) (95th percentile) | |
| | | | Cobalt (Co) | ≤ 0.05 milligrams/litre (mg/l) (95th percentile) | |
| | | | Zinc (Zn) | ≤ 0.002 milligrams/litre (mg/l) (95th percentile) | |
| Habitat | Instream | Habitat diversity should be maintained in a B ecological category. | Index of Habitat Integrity, Rapid Habitat Assessment Method and Model (RHAMM) | Instream Habitat Integrity EC = B ≥ 82% | Maintenance of ecological integrity. Present ecological state. |
| | Riparian habitat | Riparian vegetation should be maintained within B ecological category. | Index of Habitat Integrity, Vegetation Response Assessment Index | VEGRAI EC = B ≥ 82% | Maintenance of ecological integrity. Present ecological state |



3.2 Local Network Drainage Medupi Power Station

Medupi Power Station is situated in the Limpopo Plain climate zone (Kleynhans et al. 2005). This climate zone is characterized by plains and lowlands, with low to moderate relief. The vegetation consists mostly of Bushveld and Mopane Veld.

The study area is situated in the Steenbokpan area which lies in the A42J quaternary catchment. The ADF site is located to the west of Medupi Power Station. The general layout of the site in which the proposed activities will take place is shown in the google image in Figure 2 in relation to the Sandloop River.



Figure 2: Area in which activities are to be undertaken

The tributary of the Sandloop River drains from the northwest to the southeast of the existing ADF footprint. The possible impacts to surface water would therefore be the potential reduction in catchment runoff and impacts from contaminants from the ADF and associated pollution control dams. The Sandloop River drains close to licensed disposal facility, and it is for this reason that a floodline delineation exercise was required to determine the effect of the 1:100 year flood on the ADF.

3.3 Rainfall and Evaporation

3.3.1 Rainfall

Rainfall data in the area around Medupi Power Station was sourced from the Daily Rainfall extraction utility (Kunz, 2004). The rainfall stations are presented in Table 3 and can be seen in Figure 1.

Table 3: Rainfall Stations in the Lephalale Area around the Medupi Power Station

| Station | Name | Altitude (masl) | From | To | No. of Years | Distance to Medupi (km) | MAP (mm) |
|-----------|-----------------|-----------------|------|------|--------------------|-------------------------|----------|
| 0717834_W | De Dam | 825 | 1903 | 2000 | 97 (73.1% patched) | 35.416 | 372.65 |
| 0717624_P | Parrs Halt | 824 | 1903 | 2000 | 97 (61.9% patched) | 39.994 | 380.63 |
| 0717595_W | Stockport (POL) | 824 | 1903 | 2000 | 97 (35.4% patched) | 39.441 | 416.09 |
| 0718147_W | Deelkraal | 865 | 1908 | 2000 | 93 (86.9% patched) | 29.791 | 410.82 |
| 0717418_P | Dikgatlong | 834 | 1903 | 2000 | 97 (63%) | 42.811 | 457.30 |



patched)

Figure 3 shows the monthly rainfall distribution for the five rainfall stations in the Lephalale area over a period of approximately 100 years. It can be seen that the monthly rainfall is fairly uniform. The South African Weather Services (SAWS) station Stockport (POL) number 0717595_W was chosen as the station used in the study due to it being the average among the stations available and is the most reliable in terms of the number of years of observed data. Figure 4 shows the cumulative plots for the five rainfall stations. This is done to check if there are any anomalies in the recorded data and compare the data record lengths of each station.

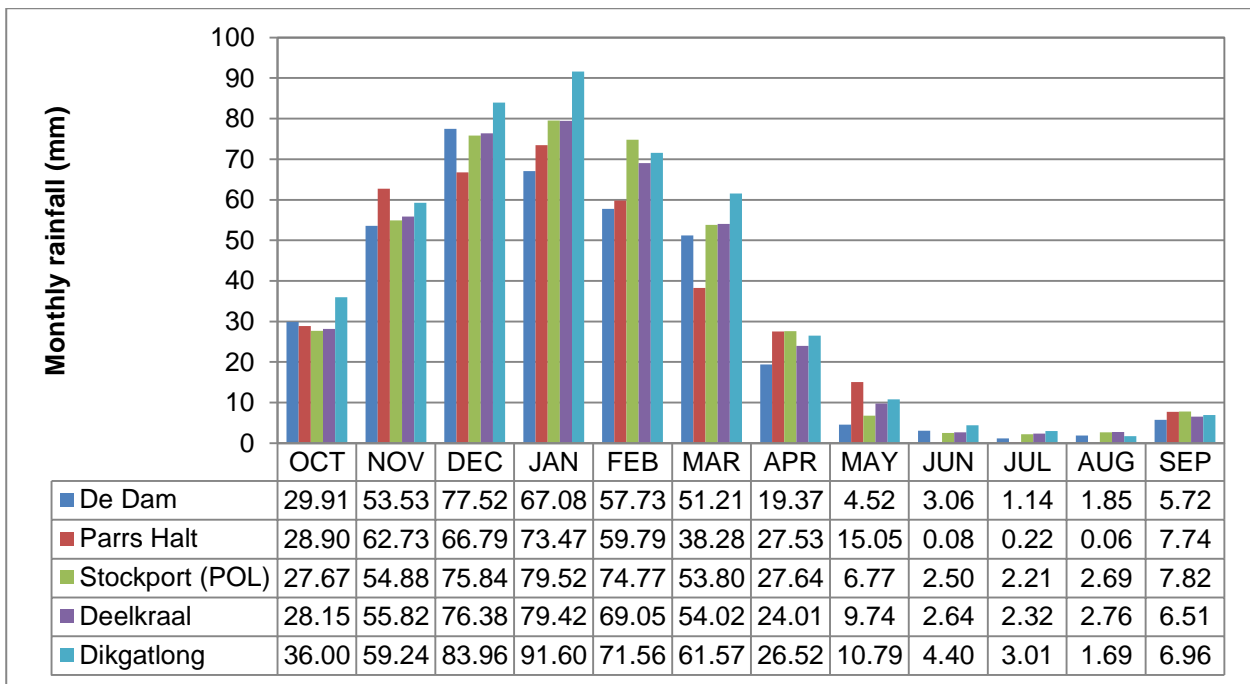


Figure 3: Monthly rainfall distribution for rainfall stations in the Lephalale area

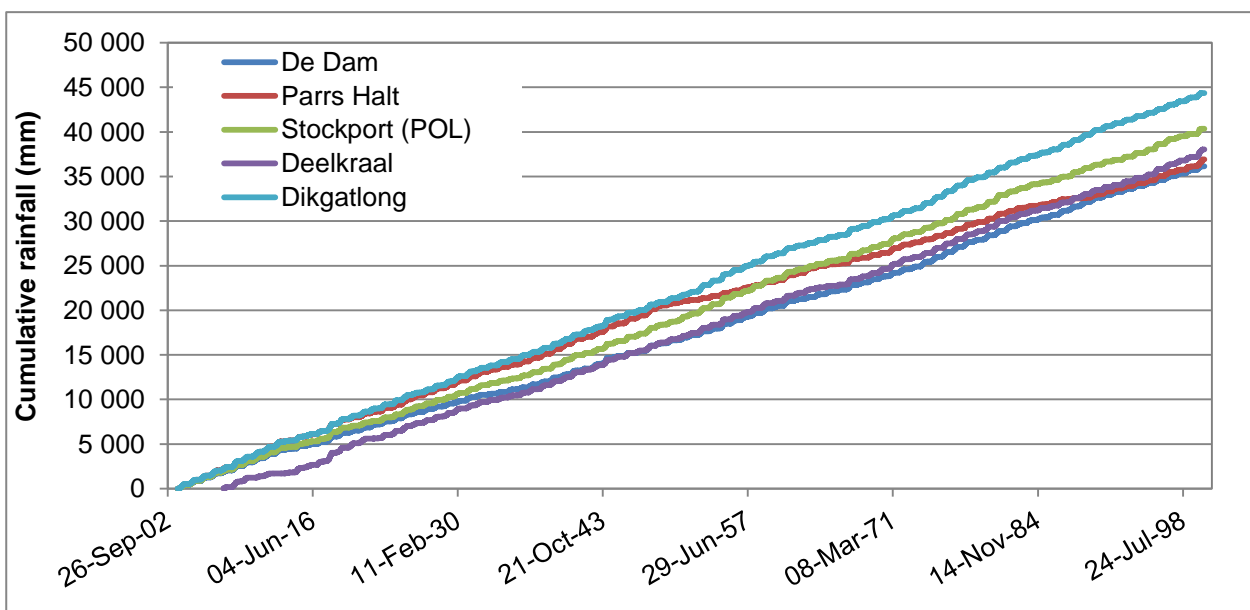


Figure 4: Cumulative rainfall for rainfall stations in the Lephalale area



Figure 5, Figure 6 and Figure 7 show the daily rainfall, monthly boxplot and the annual rainfall for the Stockport (POL) Rainfall Station respectively.

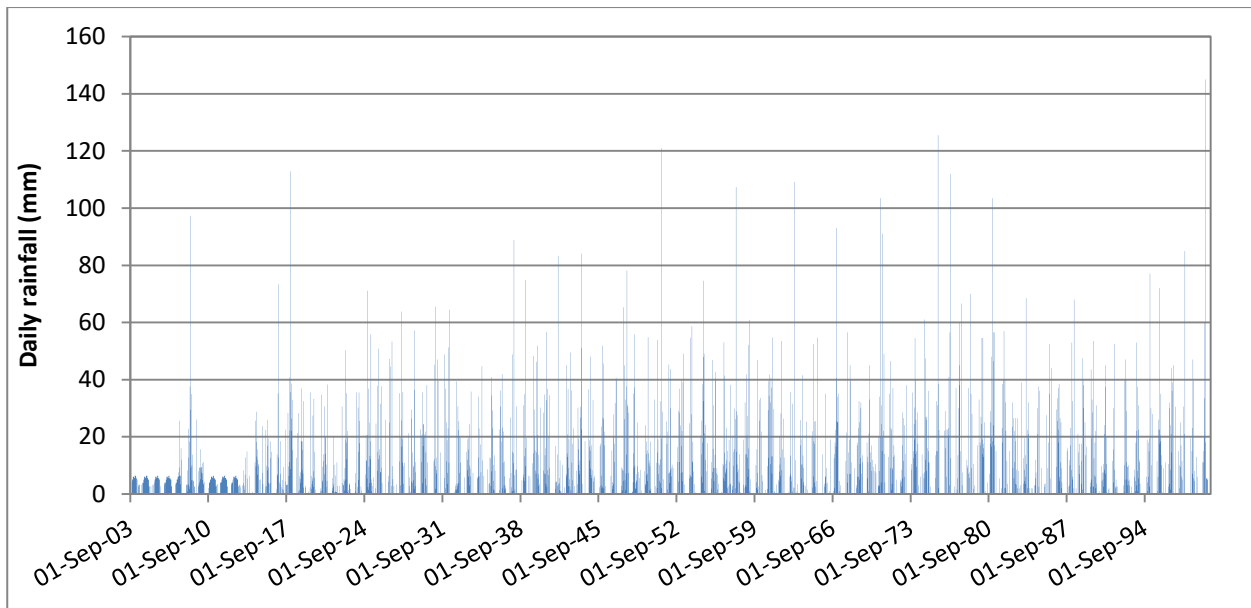


Figure 5: Daily rainfall for Stockport (POL) Rainfall Station (0717595 W)

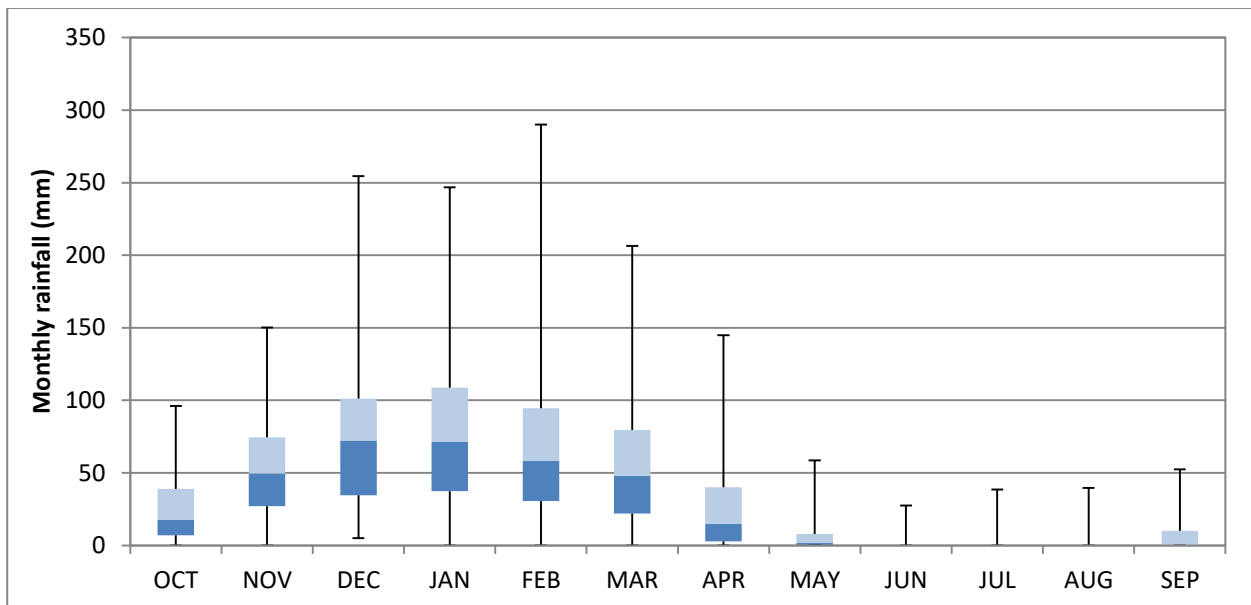


Figure 6: Monthly rainfall boxplot for Stockport (POL) Rainfall Station (0717595 W)

The boxplot in Figure 6 identifies the minimum, first quartile, median, third quartile, and maximum value in the monthly data set. It also highlights the amount of data, as a percentage, that falls below and above the 25%, 50%, and 75% mark.

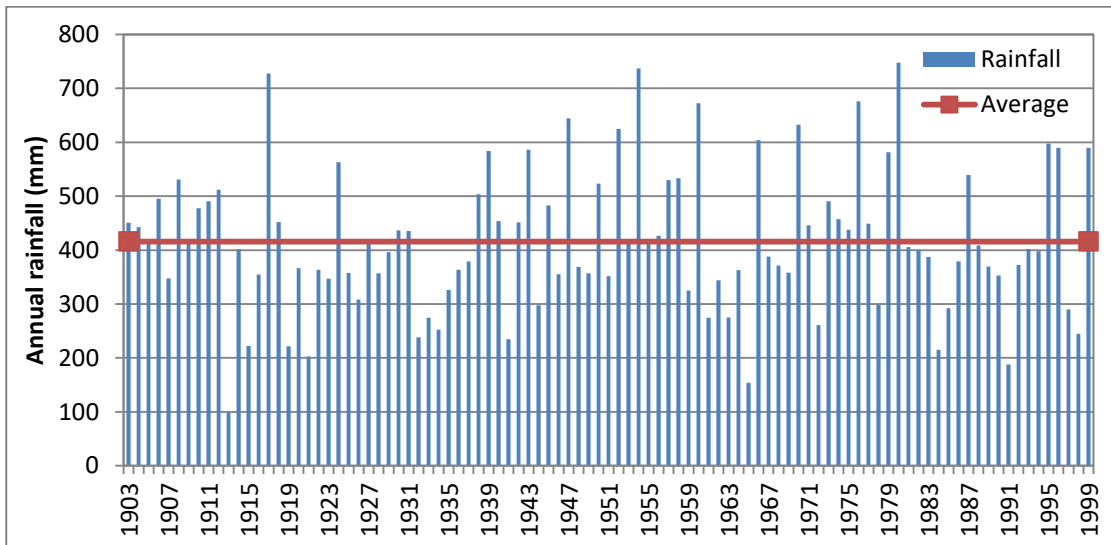


Figure 7: Annual rainfall measured at Stockport (POL) Rainfall Station (0717595 W)

The mean annual rainfall for Stockport (POL) is 416.09 mm. The lowest rainfall year was 1913 with 98.6 mm and the highest rainfall year was 1980 with 747.9 mm.

The 5, 50 and 95 percentiles of the annual rainfall totals for the rainfall station are presented in Table 4. Figure 8 shows the cumulative distribution function of the annual rainfall totals measured at the Stockport (POL) station.

Table 4: 5, 50 and 95 Percentile of the Annual Rainfall Totals

| Station number | Station name | 5 th percentile | 50 th percentile | 95 th percentile |
|----------------|-----------------|----------------------------|-----------------------------|-----------------------------|
| 0717595 W | Stockport (POL) | 209.21 | 421.70 | 636.55 |

Table 4 shows for Stockport (POL) there was:

- Less than 209 mm/annum rainfall for 5 % of the time;
- Less than 422 mm/annum rainfall for 50 % of the time; and
- Less than 637 mm/annum rainfall for 95 % of the time.

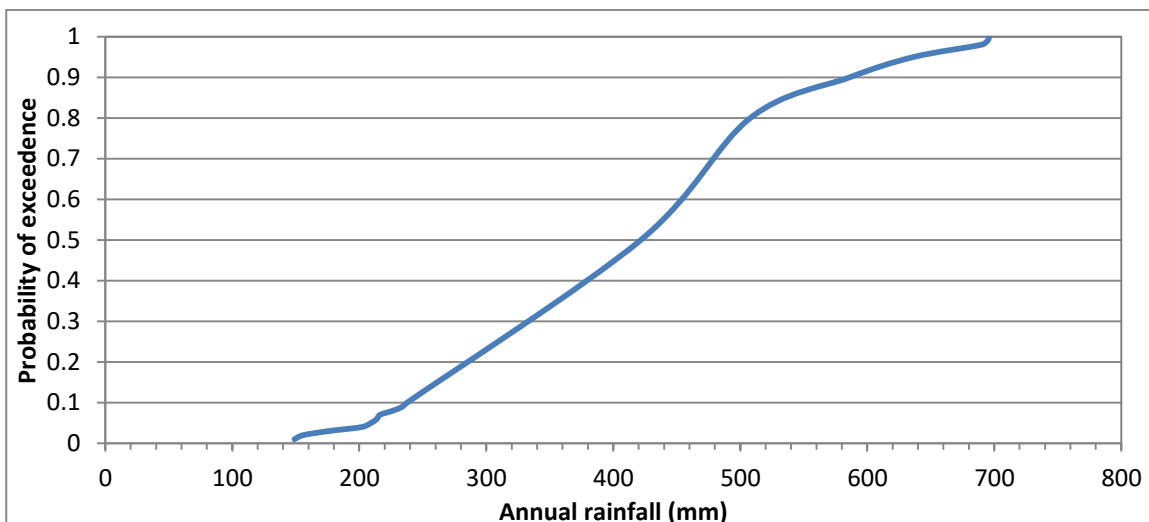


Figure 8: Annual probability curve for the Stockport (POL) Rainfall Station (0717595 W)



At the Stockport (POL) station 75 events measured more than 50 mm/day and rainfall events with more than 100 mm/day were recorded 9 times during the data period. Table 5 shows all the highest recorded rainfall events at the Stockport (POL) station.

Table 5: High Rainfall Events

| Maximum recorded daily rainfall (mm) | Date of maximum rainfall |
|--------------------------------------|--------------------------|
| 112.9 | 29 December 1917 |
| 120.9 | 22 April 1951 |
| 107.4 | 6 January 1958 |
| 109.2 | 7 April 1963 |
| 103.5 | 19 December 1970 |
| 125.5 | 11 February 1976 |
| 112 | 26 March 1977 |
| 103.5 | 6 January 1981 |
| 145 | 8 February 2000 |

The 24-hour storm rainfall gridded data for the 1:2, 1:5, 1:10, 1:20, 1:50, 1:100 and 1:200-year recurrence interval was abstracted from the database using the Design Rainfall Estimation Programme (Smithers & Schulze, 2002) from the closest rainfall station and are given in Table 6. South African Weather Services (SAWS) Rainfall station 0717595_W (Stockport POL) was used for this study. The selection of station 0717595_W was based on the fact that this is the closest station to the study area with a reliable record. The rainfall distribution on site is classified as a type 3 design rainfall distribution.

Table 6: 24 Hour Rainfall Depths for Different Recurrence Intervals (mm/day)

| Recurrence interval (years) | 1 in 2 | 1 in 5 | 1 in 10 | 1 in 20 | 1 in 50 | 1 in 100 | 1 in 200 |
|-----------------------------|--------|--------|---------|---------|---------|----------|----------|
| 24-hour rainfall depth (mm) | 61.7 | 87.1 | 105.3 | 123.9 | 149.7 | 170.3 | 192.0 |

3.3.2 Potential Evaporation

The Monthly evaporation data was available for two DWS stations namely A4E003 (23°50'34.52"S and 27°47'58.90"E some 30km South East of site), Zandpan and A4E007 Mokolo Nature Reserve @ Mokolo Dam (23°58'32.49"S and 27°43'28.89"E some 35km South East of site). The mean annual evaporation (MAE) for station A4E003 is 2 572 mm and is 2 014 mm for station A4E007. Monthly mean, minimum and maximum evaporation depths are shown in Figure 9.

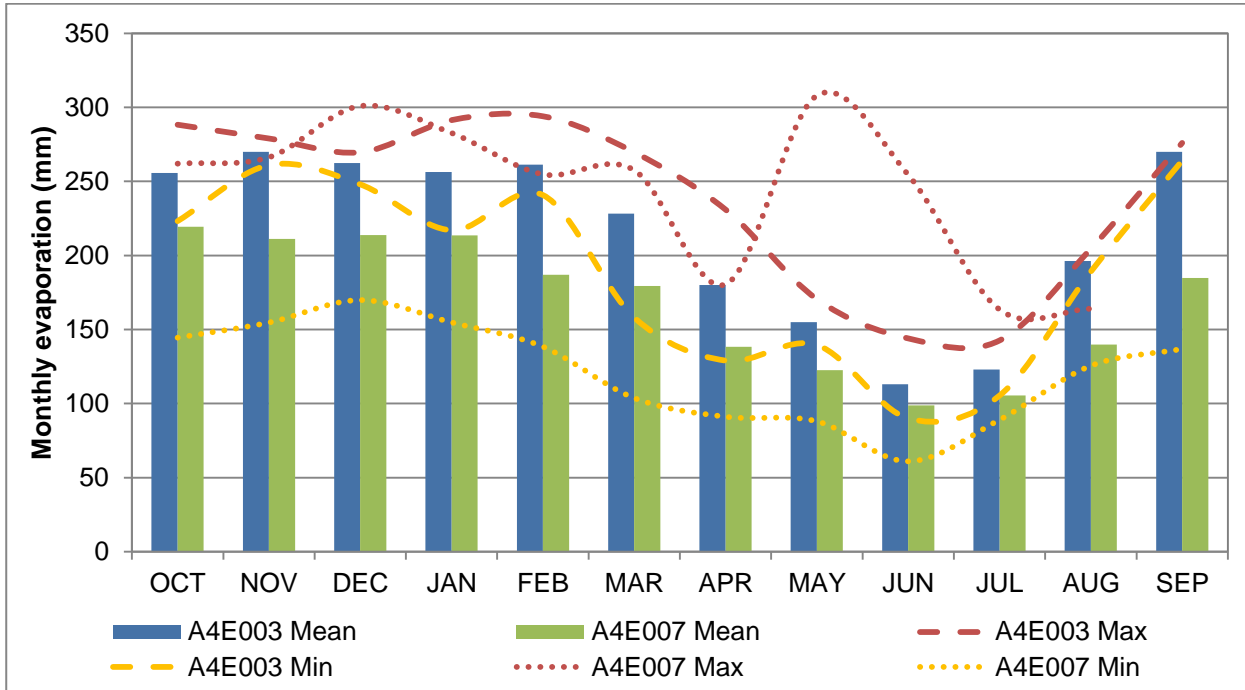


Figure 9: Monthly mean, minimum, maximum evaporation for stations A4E003 and A4E007

Figure 9 shows that the highest evaporation occurs in the summer months from September to March. This is verified in Table 7 which shows the average monthly evaporation for the two stations.

Table 7: Average monthly evaporation values for stations A4E003 and A4E007

| Month | A4E003 | A4E007 |
|--------------|--------------|--------------|
| Oct | 255.75 | 219.38 |
| Nov | 270.00 | 211.21 |
| Dec | 262.47 | 213.81 |
| Jan | 256.27 | 213.56 |
| Feb | 261.40 | 186.99 |
| Mar | 228.37 | 179.34 |
| Apr | 180.00 | 138.32 |
| May | 155.00 | 122.51 |
| Jun | 113.00 | 98.83 |
| Jul | 122.97 | 105.45 |
| Aug | 196.33 | 139.85 |
| Sep | 270.00 | 184.88 |
| Total | 2 572 | 2 014 |



3.4 Water quality and quantity

It is recommended that the water quality and water volumes on site be monitored on the surface watercourses around Medupi Power Station. The major constituents of concern would emanate from the ADF.

Fly ash contains trace concentrations of metals and other substances that are known to be detrimental to health in sufficient quantities. Potentially toxic trace elements in coal include arsenic, beryllium, cadmium, barium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, uranium, vanadium, and zinc.

Natural gypsum and FGD gypsum have the same chemical composition: calcium sulfate dihydrate ($\text{CaSO}_4 \cdot 2\text{H}_2\text{O}$). FGD gypsum production and sales encourages power producers to capture “waste” for reuse, rather than merely storing it. However, certain impurities occasionally occur with natural as well as synthetic gypsum. The impurities are generally inert and harmless and typically consist of clay, anhydrite, or limestone in natural gypsum and fly ash in synthetic gypsum, which is likely to be the case here. Each individual source must be analyzed separately to assess its particular suitability which may vary depending on purity.

Based on the potential contaminants of concern the recommended water quality programme is as follows:

- The existing (NSS) as well as proposed (Golder) water quality monitoring points are shown in Figure 10;
- The existing water quality and water volumes monitoring points are listed in Table 8 and the laboratory analysis results for samples collected at these points in November 2015 are listed in Table 10;

For this study, three monitoring points in the Sandloop River and two points on the unnamed tributary were identified and sampled. The properties of the proposed water quality monitoring locations are listed in Table 9.

- The three monitoring locations in the Sandloop River were identified to establish a baseline water quality and flow along the main watercourse;
- The remaining two monitoring sites are located on the unnamed tributary of the Sandloop River that runs to south west of the existing licensed disposal facility. The monitoring points include one upstream of the disposal facility and one downstream of the disposal facility before the confluence with the Sandloop River;
- Samples should be taken monthly or when water is present at the proposed locations. During the dry season, each monitoring site should be visited every two to three months to see if there is water that can be sampled; and
- The parameters to be analysed should include:
 - pH, Total Dissolved Solids, Electrical Conductivity, Alkalinity, Potassium, Calcium, Sodium, Chloride, Fluoride, Sulphate, Nitrate, Ammonium, Total Hardness, Metals: Arsenic, Beryllium, Cadmium, Barium, Chromium, Copper, Lead, Mercury, Molybdenum, Nickel, Selenium, Uranium, Vanadium and Zinc using ICP-MS), Orthophosphate, Total Suspended Solids, Oil and Grease

Water quality data were collected at the existing monitoring points in November 2015 by Natural Scientific Services (NSS). However, no water quality or flow data were collected during the site visit undertaken by Golder on 7th to 8th November 2016 because the proposed points were dry. Since one set of water quality results exists, limited analysis of the laboratory results can be carried out and no trends could be established.



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Table 8: Existing surface water quality and quantity monitoring sites at Medupi

| Golder Site Name | River/ Location | Latitude | Longitude | Motivation for point location |
|------------------|----------------------------|---------------|---------------|---|
| MD1 | Sandloop tributary (major) | 23°43'22.38"S | 27°29'24.49"E | Provide water quality on major tributary upstream of Eskom operation. |
| MD2 | Sandloop tributary | 23°43'54.09"S | 27°30'51.95"E | Provide water quality and quantity after tributary passes Site 13 (existing ADF). |
| MD3 | Site 2 (proposed) | 23°44'50.52"S | 27°30'16.55"E | Provide water quality at proposed Site 2. |
| MD4 | Site 12 (proposed) | 23°43'38.15"S | 27°31'42.38"E | Provide water quality at proposed Site 12. |
| MD5 | Sandloop tributary (minor) | 23°44'20.34"S | 27°32'55.28"E | Provide water quality on minor tributary downstream of Eskom operation. |
| MD6 | Sandloop River | 23°44'45.55"S | 27°34'19.61"E | Establish water quality on the Sandloop River. |

Table 9: Proposed surface water quality and quantity monitoring sites at Medupi

| Golder Site Name | River/ Location | Latitude | Longitude | Motivation for point location |
|------------------|--|---------------|---------------|--|
| WQ1 | Sandloop River (upstream) | 27°26'34.96"E | 23°47'42.65"S | Establish baseline water quality data furthest upstream Sandloop River. |
| WQ2 | Sandloop tributary (major, upstream) | 27°29'19.53"E | 23°43'19.53"S | Provide water quality on major tributary upstream of Site 13 (ADF). |
| WQ3 | Sandloop River (central) | 27°30'36.07"E | 23°45'38.27"S | Establish baseline water quality and flow data in the Sandloop River across Eskom operation. |
| WQ4 | Sandloop tributary (major, downstream) | 27°32'10.80"E | 23°44'42.77"S | Provide water quality and flow on major tributary downstream of Site 13 (ADF). |
| WQ5 | Sandloop River (downstream) | 27°34'10.40"E | 23°44'38.95"S | Establish baseline water quality data furthest downstream Sandloop River. |

Table 10 shows the results for water quality samples collected at MD1 to MD6 (existing monitoring points) in November 2015. These results are compared against the proposed RQOs (Table 2) for the Sandloop.

Table 10: Water quality data from Waterlab (Pty) Ltd for the grab sample taken in November 2015 against the water quality component of the RQOs proposed for the Sandloop

| Sample ID | Units | RQO or WQG [#] | MD1 | MD2 | MD3 | MD4 | MD5 | MD6 |
|--------------------------------|------------------------|-------------------------|------------|------------|------------|------------|------------|------------|
| | | | 28/11/2015 | 28/11/2015 | 28/11/2015 | 28/11/2015 | 28/11/2015 | 28/11/2015 |
| pH @ 25°C | - | 6.5 – 8.5 | 7.20 | 7.10 | 7.00 | 7.10 | 7.00 | 6.90 |
| Electrical Conductivity | mS/m | ≤ 55 | 12.70 | 29.70 | 7.20 | 9.40 | 24.00 | 15.20 |
| Total Dissolved Solids @ 180°C | mg/L | ≤ 260* | 109.00 | 218.00 | 56.00 | 54.00 | 129.00 | 89.00 |
| Total Alkalinity | mg/L CaCO ₃ | - | 28.00 | 32.00 | 16.00 | 20.00 | 20.00 | 20.00 |
| Total Hardness | mg/L CaCO ₃ | ≤ 100 | 40.00 | 80.00 | 17.00 | 27.00 | 43.00 | 36.00 |
| Chloride as Cl | mg/L | ≤ 100 | 8.00 | 7.00 | 6.00 | 4.00 | 17.00 | 13.00 |



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| Sample ID | Units | RQO or WQG [#] | MD1 | MD2 | MD3 | MD4 | MD5 | MD6 |
|-----------------------------|-------|-------------------------|-------|-------|-------|-------|-------|-------|
| Sulphate as SO ₄ | mg/L | ≤ 400 | 43.00 | 93.00 | 23.00 | 16.00 | 46.00 | 32.00 |
| Nitrate as NO ₃ | mg/L | ≤ 0.1 | <0.1 | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 |
| Nitrate as NO ₂ | mg/L | - | 0.06 | <0.05 | <0.05 | <0.05 | 0.20 | <0.05 |
| Ortho Phosphate as P | mg/L | ≤ 0.05 | 1.00 | <0.1 | 0.40 | <0.10 | <0.10 | <0.10 |
| Ammonia NH ₃ | mg/L | ≤ 0.007 | 2.80 | 0.20 | 1.60 | 0.30 | 0.10 | 0.30 |
| Sodium Na | mg/L | ≤ 70 | 2.00 | 17.00 | 1.00 | 4.00 | 20.00 | 7.00 |
| Potassium as K | mg/L | ≤ 50 | 18.80 | 14.20 | 9.20 | 8.60 | 19.40 | 12.30 |
| Calcium as Ca | mg/L | ≤ 32 | 8.00 | 19.00 | 4.00 | 6.00 | 9.00 | 8.00 |
| Magnesium as Mg | mg/L | ≤ 30 | 4.00 | 8.00 | 1.00 | 3.00 | 5.00 | 4.00 |

* calculated from EC*6.5; [#]South African Water Quality Guideline for ecosystems/ domestic use/ irrigation (strictest)

A summary of the water quality results (as per Table 10) indicates that the only concern was noted for ammonia, the likely source being from livestock.



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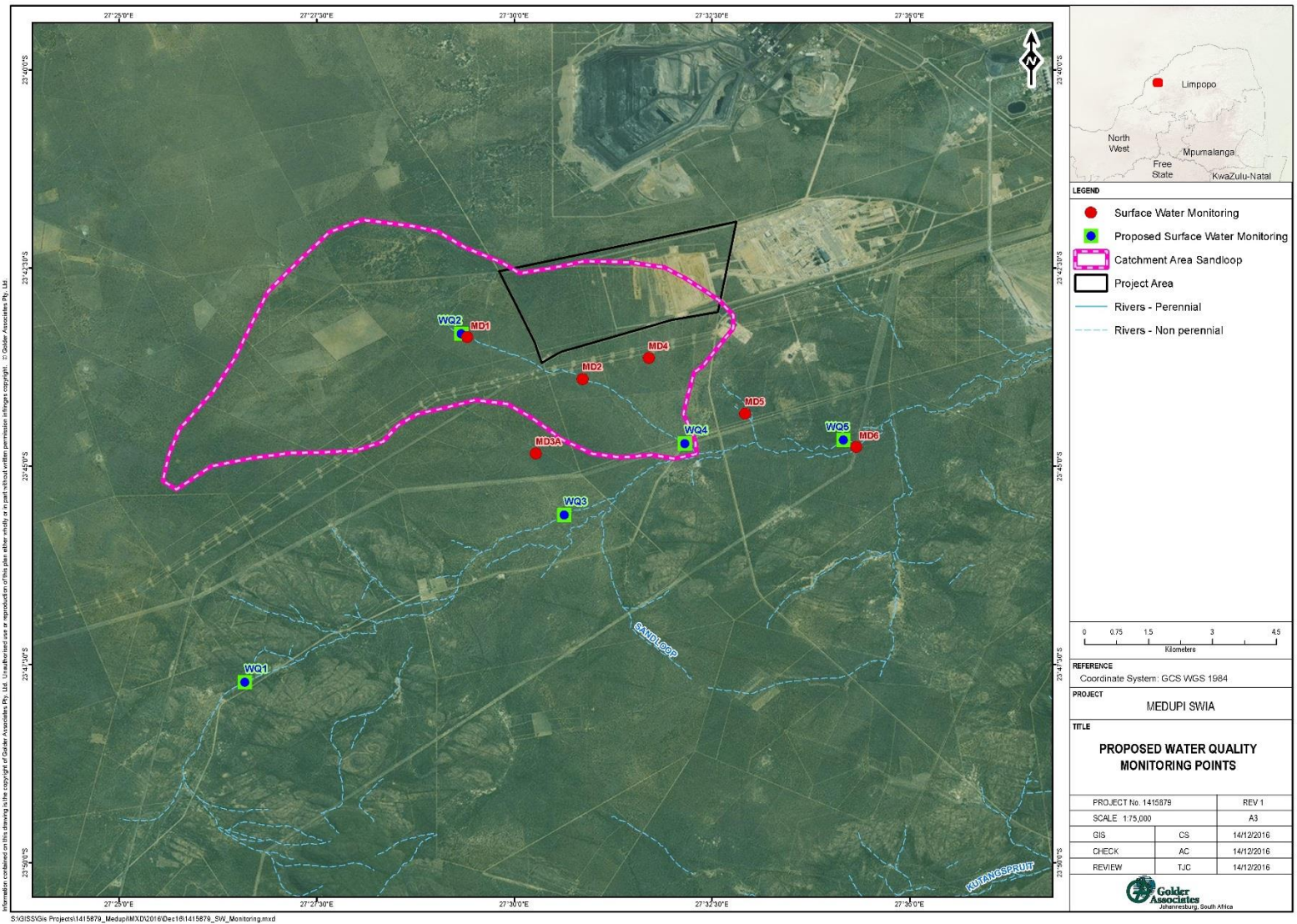


Figure 10: Medupi Power Station study area with existing and proposed water quality monitoring points



4.0 EXISTING STORM WATER MANAGEMENT SYSTEM (SWMS)

4.1.1 Overview

During the site visit conducted by Golder, a storm water management system was identified. The storm, water management system is aimed at mitigating the impact of the existing Medupi project on downstream water quality.

A General layout of the existing ADF and storm water management philosophy is provided in Appendix B. The existing water management system at Medupi includes:

- A dirty water management system to ensure that polluted water the power station and its associated infrastructure, including the existing ADF, as well as sediment-laden runoff from disturbed areas is separated from clean area runoff and that it is collected in Pollution Control Dams (PCD); and
- A clean water management system to divert water undisturbed by the power station's operations around the disturbed project footprint.

Additional facilities proposed and phased in over several years include three additional pollution control dams on the southern side of the ADF. All undeveloped, natural veld areas are clean and assumed to be free draining.

4.1.2 Water Conveyance and Storage

The following paragraphs describe a typical storm water management system (SWMS) as prescribed by Regulation 704 (GN704) and Best Management Practices (BMP's) and information gathered from the client during the site visit.

Clean and Dirty Water Areas

For the existing footprint at Medupi, clean and dirty catchments are delineated for the surface works using the site infrastructure layout plan and most recent aerial imagery. All flows from polluted surfaces are contained in dirty water systems. Only the surface areas which are anticipated to be impacted by the either clean or dirty water infrastructure as presented by the infrastructure layout plan and aerial imagery have been estimated. In all other areas around the power plant the natural pathways will be followed, ultimately routing surface water into the nearby watercourses. As such, these areas should not contain any infrastructure or workings which would be defined as dirty, unless additional mitigation has been included.

Clean Water Diversions

The Storm Water Management Plan (SWMP) includes typical upstream clean water diversions consisting of berms and cut-off trenches. Clean water diversion berms and cut-off trenches are designed to divert upstream clean water around dirty water generating areas (i.e. intercepting clean water runoff and diverting this water around mining activities). These diversions should be sized to cater for the 1:50 year flood event (with dimensions finalised during the detailed design phase of the project).

Dirty Water Containment

As per the clean water diversions, dirty water containment systems are designed to ensure dirty water generated within the footprint of Medupi is contained on site. These systems contain a channel and a storage component. Lining of the dirty water diversions is included to prevent seepage of any pollutants into the soil profile and subsequent percolation into groundwater. These diversions are typically sized to cater for a minimum of the 1:50 year flood event. Dirty water areas should be managed as a closed and separate system that is regulated by a collection point or Pollution Control Dam (CPD).

4.1.3 Findings of SWMS

The following can be concluded from the existing storm water management system at Medupi:

- No surface watercourses exist within the Medupi Power Station footprint;



- Surface access water generated from polluted areas during rainfall events within the power station's dirty footprint is contained on site by means of a dirty water management system that comprises a series of lined drains and pollution control dams (PCD's), which will be further developed as necessary (Appendix B); and
- Likewise, runoff resulting from unpolluted areas is channelled back into the environment via a series of open storm water channels.

Figure 11 and Figure 12 show a lined dirty water containment channel alongside the ADF at Medupi while Figure 13 shows an unlined clean water diversion channel on the periphery of the power station.



Figure 11: Concrete-lined dirty water containment drain alongside ADF at Medupi



Figure 12: Concrete-lined dirty water containment channel draining into PCD at Medupi



Figure 13: Unlined clean water diversion channel at Medupi



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Knight Piésold undertook an assessment of the storm water management system in October 2017. The following regulations were considered:

- GN704: National Water Act, 1998 (Act No. 36 of 1998) Regulations on use of water for mining and related activities aimed at the protection of water resources; and
- Liner Regulations: Liner containment barrier systems. National Environmental Management Act (Act 59 of 2008). NEMWA Regulations R634, R635 and R636.

The results of the assessment indicate that the post-development flood peaks (after construction of the FGD area) are less than the pre-development flood peaks. It is noted in the report that this was due to the conservative approach adopted in the pre-development scenario as more development of the catchment was foreseen. This was done to allow for substantial development within the terrace area without having to increase the storm water system capacity once the final infrastructure layout is developed. The results indicated that approximately 35% of the total conveyance capacity is utilized. Two alternatives were considered for storm water management post-development of the FGD area and are described in the design report (Knight Piésold, 2017).

The recommendations from the report are that based on the re-designation of the catchments areas from clean to dirty (see Figure 14 and Figure 15), 20% of the total dirty water catchment areas will now be added to the dirty water system. It is therefore anticipated that the existing Dirty Water Dam (102 00 m³ capacity) will have insufficient capacity to store the new dirty water runoff volumes (Figure 16). Additional dirty water storage will be required. This was not been sized as it was not part of the scope. The Dirty Water Dam capacity would have to be validated using a water balance so as to take into account the demands on the Dam. The 9% reduction in clean water areas indicates that the Clean Water Dam (133 400 m³ capacity) will have sufficient capacity to cope with the proposed FGD infrastructure.

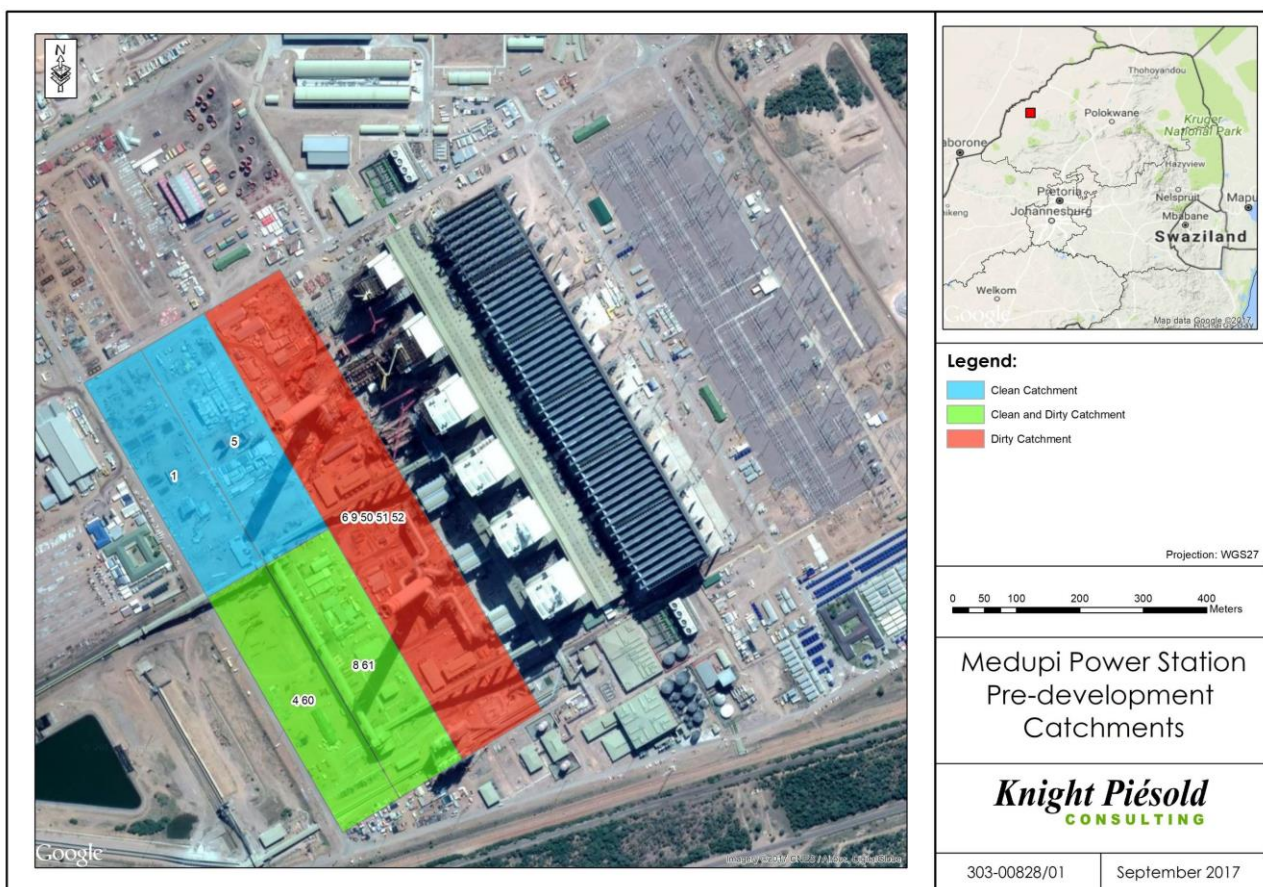


Figure 14: Existing clean and dirty water catchments in the FGD area (Knight Piésold, 2017)



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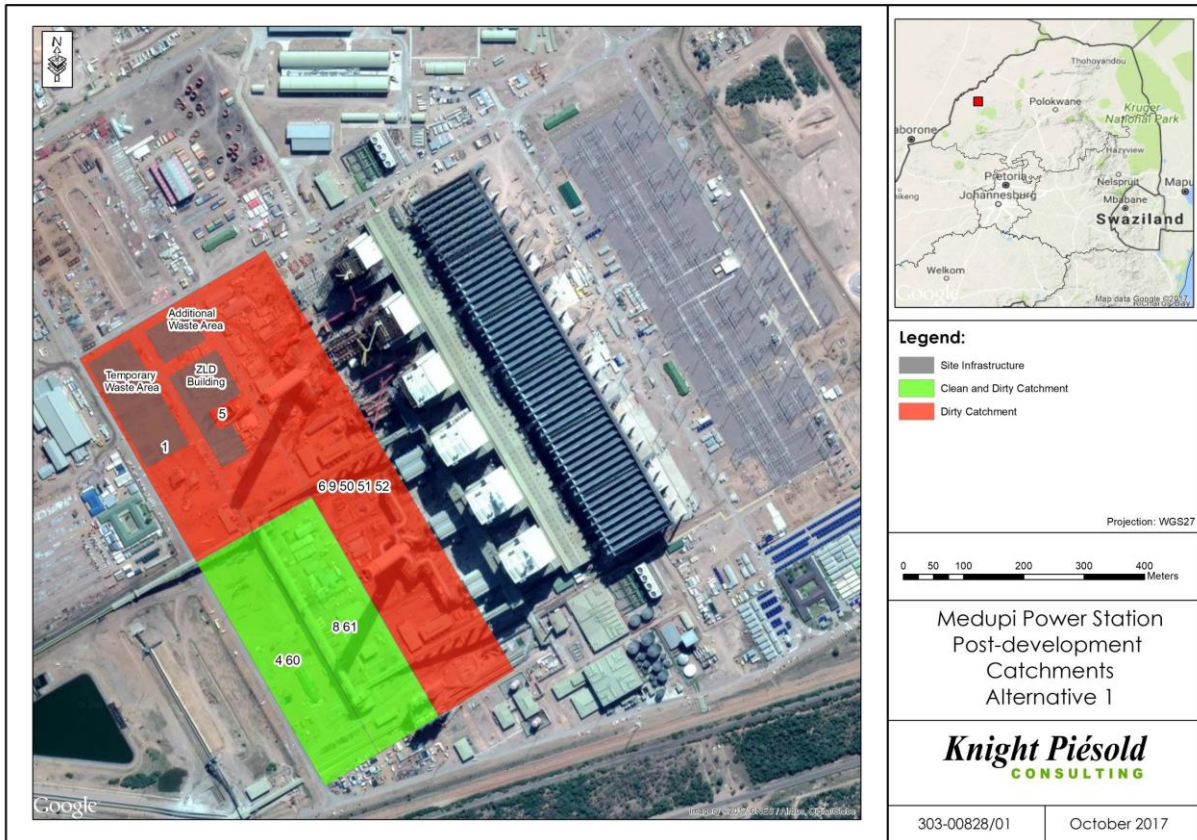


Figure 15: Re-designated catchment areas 1 and 5 from clean to dirty

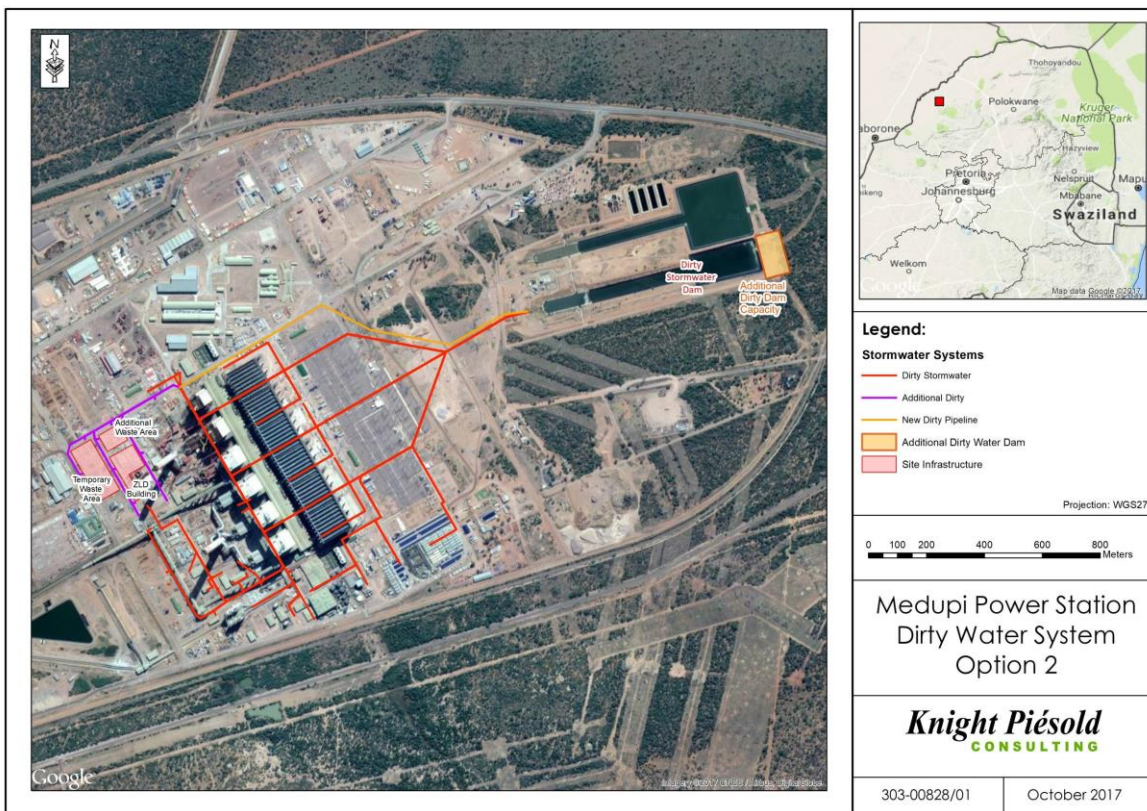


Figure 16: Dirty storm water system recommendation (Knight Piésold, 2017)



5.0 WATER BALANCE

5.1 Overview

A numerical site-wide water balance model has been developed for the existing operations at Medupi Power Station in order to assess the effectiveness of the power stations' water management system. However, a copy of this study has not been obtained at this stage and therefore no further reference can be made or conclusions drawn from it.

It is nonetheless recommended that a revision of this water balance study be carried out to include the FGD retrofit project as well as the proposed expansion of the existing ADF.

5.2 Findings of Water Balance Study

The existing water balance should be made available for detailed analysis. The analysis would typically require a review of the following:

- Modelling methodology highlighting the methodology adopted in modelling the system;
- Model configuration detailing how the model has been configured to simulate the operations of all major components of the water management system. This will include all operating rules of the water balance simulation;
- Water demands within the modelled system;
- Water sources (on and off-site) used in the model;
- Catchment/ site runoff for all the predicted catchment areas that drain to various water management dams and storages;
- Raw water (water imported from the local water authority) that is required to sustain the nominated design production rate and associated operational demands of the project; and
- Detailed analysis of the simulation results, including plant complex storage inventory, offsite water requirements, uncontrolled spills from site water storages, and the overall water balance within the study area.

6.0 FLOODLINE DETERMINATION

The 1:50 and 1:100 year floodline for the streams that could be impacted by the mine site were determined and delineated to ensure safety of mine facilities and maintenance of riparian zones.

6.1 Methodology used to determine the floodline

As per scope of work, the 1:50 and 1:100 year floodline for the Sandloop River tributary which drains adjacent to the existing licensed disposal facility was to be determined. A floodline assessment was required to determine the risk to the proposed ADF site and associated infrastructure from the 1:50 and 1:100 year flood peak. The floodlines were calculated using the HEC-RAS model, which determines the water surface elevations for the 1:50 and 1:100 year peak flow using an energy balance in which the friction loss is estimated using the Manning hydraulic equation. The floodlines of the Sandloop River tributary running in the vicinity of the proposed ADF footprint were determined. The following method was used for the determination of the floodlines:

- The site was visited to assess the site specific hydrological and hydraulic conditions;
- The catchment area of the Sandloop River tributary was delineated based on a 0.5m contour data
- Rainfall data as described in 3.3 was used as used as input into this section;
- A flood peak analysis was undertaken to determine the 50 and 100 year recurrence interval flood peaks for the watercourses within the mining boundary using the Rational Method as described in the SANRAL Drainage Manual (South African National Roads Agency Limited, 2006);



- Cross-sections were taken from the available topographical information. The extent and locations of the cross-sections along the modelled streams and tributaries is shown in Figure 18.
- Dimensions of the railway river crossing as well as other river obstructions such as culverts were determined during the site visit. These were used as input into the hydraulic model;
- The flood peaks and the survey data supplied by the client for the study area were used as inputs to the HEC-RAS backwater programme to determine the water surface elevations for the 1:50 and 1:100 year flood peak; and
- The floodlines were plotted on the available mapping.

6.2 Limitations and assumptions

The following limitations and assumptions have been made in this specialist study:

- No flow and rainfall data against which the runoff calculations might be calibrated were available. The runoff volumes were therefore calculated theoretically;
- Since there is very limited flow data available for a precise estimation of the roughness coefficients, the Manning's 'n' coefficients were estimated by comparing the vegetation and nature of the channel surfaces to published data (Webber, 1971), therefore slightly conservative estimations were adopted.

6.3 Rainfall, sub-catchments and flood peak calculations

The Medupi Ash Disposal Facility is largely located in the Sandloop River tributary sub-catchment. The total drainage area of Sandloop River tributary catchment was delineated into a sub-catchment based on the topography of the area as shown in Figure 17. From initial site investigations, it would seem that the key watercourse for floodline delineation and impact assessment would be the NFEPA to the south western corner of the ADF site. The tributary flows in a south-westerly direction near the ADF, and its catchment is most likely to be affected by the ADF site. The total drainage area of the Sandloop River tributary is small enough for one sub-catchment based on the topography of the area.



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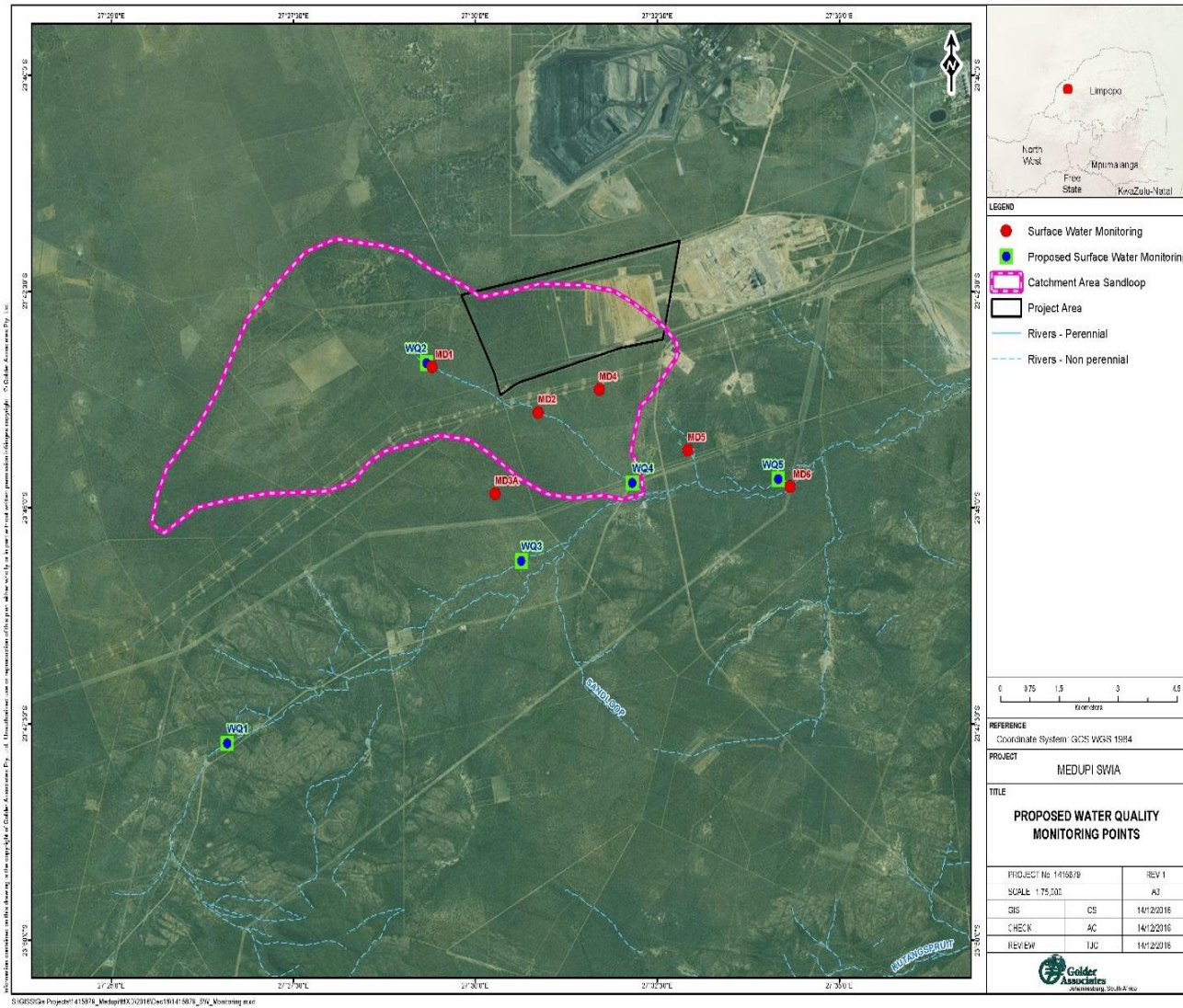
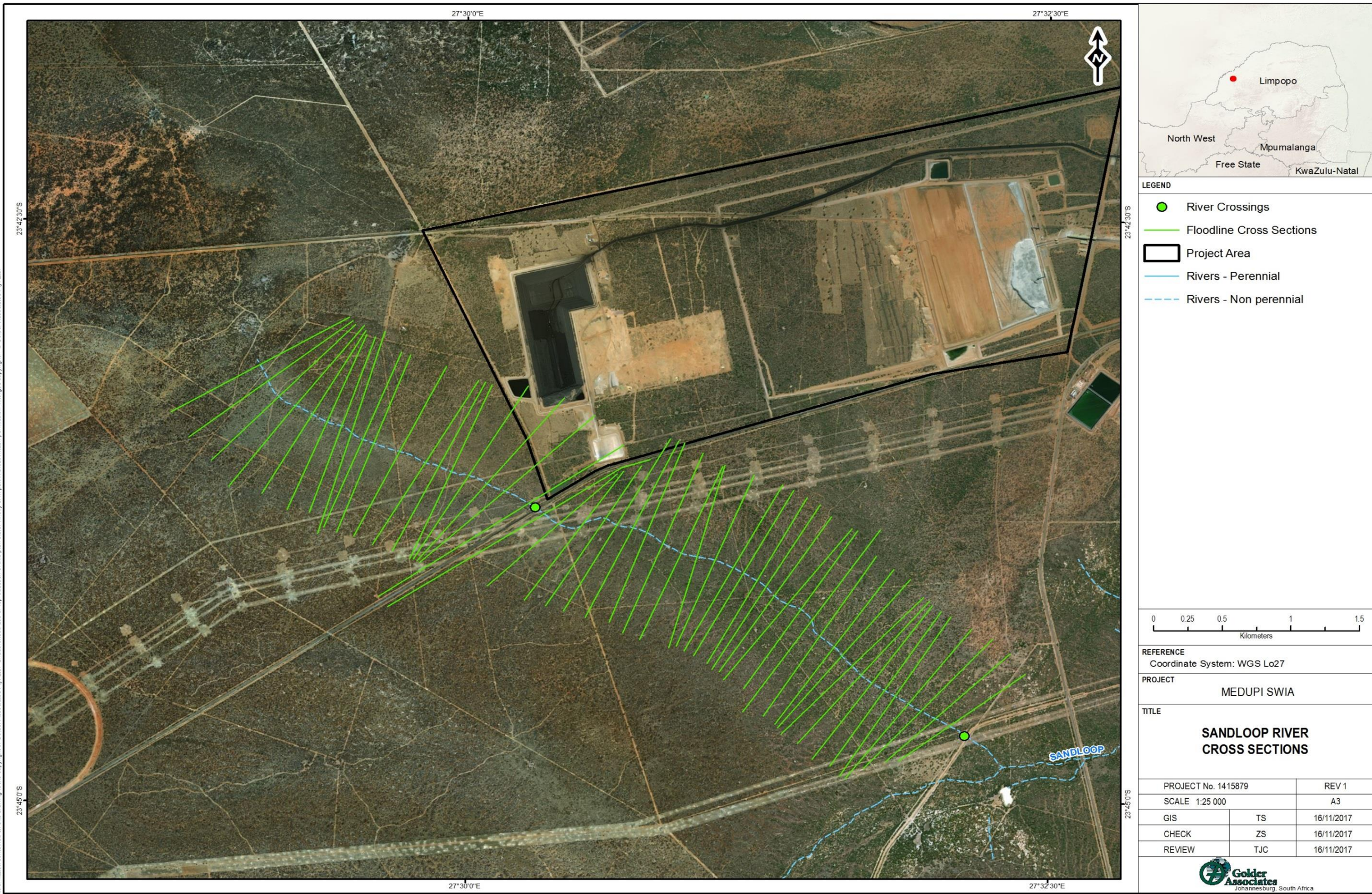


Figure 17: Discretization of Sandloop River tributary drainage area into sub-catchment



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Figure 18: The extent and locations of the cross-sections along the modelled stream



Table 11: 100 year 24 hour storm rainfall depth used as input

| Return Period (years) | 1 in 50 | 1 in 100 |
|-----------------------|---------|----------|
| Rainfall Depth (mm) | 149.7 | 170.3 |

The drainage area of the Sandloop River Tributary was delineated into one sub-catchment based on the topography of the area. The catchment boundary is shown in Figure 17. The Rational Method using Point Precipitation (RM-PP) was applied to the Sandloop River tributary sub-catchment to estimate the 1:100 year flood peak. The rational method considers the entire drainage area as a single unit and estimates the peak discharge at the most downstream point of that area. Rainfall intensity is an important parameter in the calculation of the peak flow; this is because uniform aerial and time distributions of rainfall have to be assumed. The sub-catchment characteristics used in applying the rational method are shown in Table 12.

Table 12: Sub-catchment characteristics used in the Rational Method for the Sandloop River tributary

| Name | Catchment Area (km ²) | Stream Length (m) | 10-85 Slope (m/m) | Time of Concentration (hours) |
|------|--------------------------------------|----------------------|----------------------|----------------------------------|
| ST1 | 44.67 | 6631.94 | 0.004 | 2.43 |

The rational method was used to calculate the 1:100 year peak discharge for the Sandloop River Tributary. The results are listed in Table 13.

Table 13: Computed 100 year flood peak for the Sandloop River tributary sub-catchment

| Catchment | Peak Flow (m ³ /s) | |
|-----------|-------------------------------|---------------|
| | 1 in 50 year | 1 in 100 year |
| ST1 | 144.94 | 187.57 |

The resulting floodline is mapped in Figure 19. For the modelled event, the maximum estimated hydraulic depth is 2.46m and the maximum expected velocity 1.72 m.s⁻¹. The road culvert has a restrictive effect on the flow of the stream when both the 1 in 50 and 1 in 100 flood conditions exist, hence some localised damming up at the culvert inlet can be expected. Figure 20 illustrates the encroachment of the PCD into the floodline.

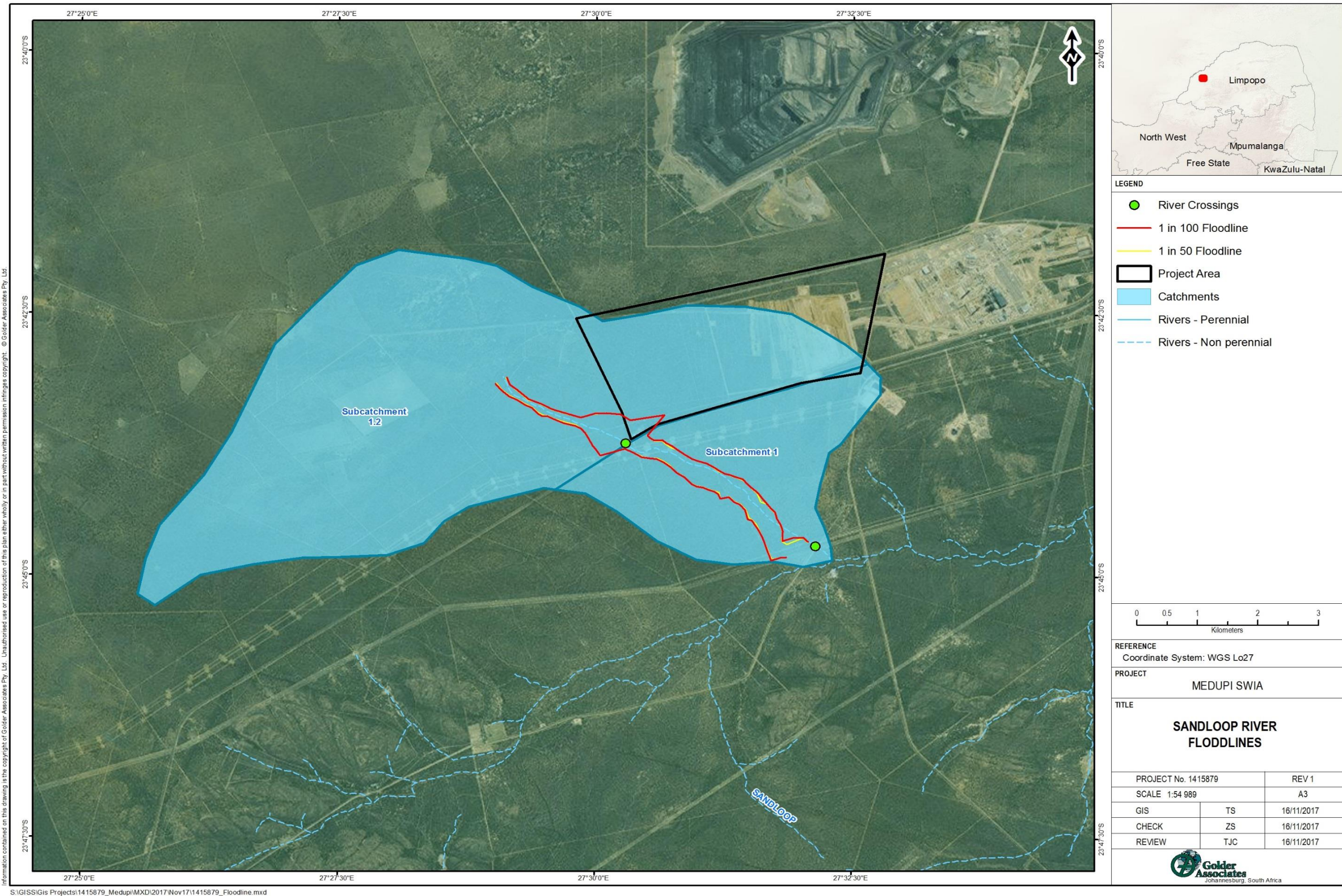


Figure 19: The 1 in 100 year recurrence interval floodline for Sandloop River Tributary



Figure 20: The 1 in 100 year recurrence interval floodline for Sandloop River showing encroachment of the PCD into the floodline



7.0 SURFACE WATER IMPACT ASSESSMENT

In investigating the receiving environment of the Medupi Power Station footprint in terms of surface water, particularly the existing licensed disposal facility, the Sandloop River and its tributaries in the vicinity of the power station were mainly considered.

7.1 Potential Surface Water Impacts

The potential surface water impacts have been assessed for the following activities:

- Construction and operation of the FGD system including the Zero Liquid Effluent (ZLD) Plant and temporary waste storage area within the Medupi Power Station Footprint;
- Construction and operation of the railway yard, limestone and gypsum handling facilities and two diesel storage facilities between the Medupi Power Station and existing ADF; and
- Disposal of ash and gypsum together on the existing ADF, including temporary disposal of salts and sludge; this will necessitate height changes to the existing ADF from 60m to 72m.

The potential surface water impacts from the project, both direct and indirect, are summarised in Table 14. In summary these potential impacts contribute to overall surface water impacts and include:

- Change in surface water catchment areas;
- Changes in surface water quality;
- Change in surface water runoff patterns;
- Erosion; and
- Potential to require off-site water supplies.

If not mitigated, the potential surface water quality impacts will ultimately affect the downstream water users. It should be noted that the Sandloop River and its tributaries are generally downstream of Medupi and the topography around the study area is such that runoff generated at Medupi drains towards the Sandloop River and its tributaries. This potentially polluted water will flow towards downstream users via the river system.

Table 14: Summary of potential surface water impacts with respect to Medupi Power Station

| Major aspect | Key Environmental Issues / Potential Impacts |
|--|---|
| <ul style="list-style-type: none"> ■ Changes in surface water catchment areas | <ul style="list-style-type: none"> ■ Disruption and reduction in land area due to construction and operation of FGD infrastructure, the railway yard and limestone and gypsum handling facilities, and disposal of ash and gypsum to the existing ADF, will be very limited due to the fact that the areas in which these facilities are to be located are within the existing Medupi operations footprint, and no additional areas will be utilised. The catchment areas that feed the Sandloop will therefore not be further impacted. |
| <ul style="list-style-type: none"> ■ Changes in surface water quality | <ul style="list-style-type: none"> ■ Poor quality runoff during construction and operation of the FGD retrofit; ■ Poor quality runoff during construction and operation of the railway yard and limestone and gypsum handling facilities; due to: <ul style="list-style-type: none"> ■ Possible fuel and lubricants spillage from equipment and other chemical spills; and ■ Inadequate storm water management (design and operation/ maintenance) resulting in poor quality runoff from disposal of ash and gypsum to the existing ADF and spillages from pollution control dams. |
| <ul style="list-style-type: none"> ■ Change in surface | <ul style="list-style-type: none"> ■ Increased runoff due to vegetation and veld removal decreases infiltration |



| | |
|-------------------------------|--|
| water runoff | into soil which may impact on downstream water users; <ul style="list-style-type: none">■ Increased runoff due to large concrete terraces and roads; and■ Potential to increase Sandloop River flood levels and flood extent. |
| ■ Erosion | <ul style="list-style-type: none">■ Erosion on site and surrounding areas may be increased due to site clearance of vegetation and veld; and■ Un-lined storm water management channel erosion. |
| ■ Off-site water requirements | <ul style="list-style-type: none">■ The potential need to import raw water in the case of a shortfall of water captured on site during dry periods. |

7.2 Impact Assessment Methodology

The Impact Assessment Methodology provided to Golder by Zitholele was used for the surface water impact assessment. The impacts will be ranked according to the methodology described below. Where possible, mitigation measures will be recommended. The impact assessment methodology makes provision for the assessment of impacts against specific criteria discussed below.

7.3 Key Definitions

The following key definitions relate to Impact Assessment Rating:

“Existing” Impact

- These are current activities that potentially have an impact on the surface water resources within the study area;
- These are baseline impacts before the proposed construction and operation of the FGD system within the Medupi Power Station Footprint; the construction and operation of the railway yard and limestone and gypsum handling facilities between the Medupi Power Station and existing ADF; and disposal of ash and gypsum together on the existing ADF;
- “Existing” excludes the proposed project for which authorisation is required.

“Cumulative” Impact

- These impacts include both the ‘existing’ activities as well as activities associated with the proposed project that potentially have an impact on the surface water resources within the study area;
- The project activities include the construction and operation of the FGD system within the Medupi Power Station Footprint; the construction and operation of the railway yard and limestone and gypsum handling facilities between the Medupi Power Station and existing ADF; and disposal of ash and gypsum together on the existing ADF; and
- The transportation of sludge and salts to an existing licensed facility

“Post-mitigation” Impact

- This impact rating takes into consideration the “cumulative” impacts after the proposed mitigation measures have been effectively implemented;
- The project activities are the construction and operation of the FGD system within the Medupi Power Station Footprint; the construction and operation of the railway yard and limestone and gypsum handling facilities between the Medupi Power Station and existing ADF; and disposal of ash and gypsum together on the existing ADF, and
- The transportation of sludge and salts to an existing licensed facility.



7.4 Nature of the impact

Each impact should be described in terms of the features and qualities of the impact. A detailed description of the impact will allow for contextualisation of the assessment.

7.5 Extent of the impact

Extent intends to assess the footprint of the impact. The larger the footprint, the higher the impact rating will be. The table below provides the descriptors and criteria for assessment.

Table 15: Criteria for the assessment of the extent of the impact

| Extent Descriptor | Definition | Rating |
|-------------------|---|--------|
| Site | Impact footprint remains within the boundary of the site. | 1 |
| Local | Impact footprint extends beyond the boundary of the site to the adjacent surrounding areas. | 2 |
| Regional | Impact footprint includes the greater surrounds and may include an entire municipal or provincial jurisdiction. | 3 |
| National | The scale of the impact is applicable to the Republic of South Africa. | 4 |
| Global | The impact has global implications | 5 |

7.6 Duration of the impact

The duration of the impact is the period of time that the impact will manifest on the receiving environment. Importantly, the concept of reversibility is reflected in the duration rating. The longer the impact endures, the less likely it is to be reversible. See Table 16 for the criteria for rating duration of impacts.

Table 16: Criteria for the rating of the duration of an impact

| Duration Descriptor | Definition | Rating |
|---|--|--------|
| Construction / Decommissioning phase only | The impact endures for only as long as the construction or the decommissioning period of the project activity. This implies that the impact is fully reversible. | 1 |
| Short term | The impact continues to manifest for a period of between 3 and 5 years beyond construction or decommissioning. The impact is still reversible. | 2 |
| Medium term | The impact continues between 6 and 15 years beyond the construction or decommissioning phase. The impact is still reversible with relevant and applicable mitigation and management actions. | 3 |
| Long term | The impact continues for a period in excess of 15 years beyond construction or decommissioning. The impact is only reversible with considerable effort in implementation of rigorous mitigation actions. | 4 |
| Permanent | The impact will continue indefinitely and is not reversible. | 5 |

7.7 Potential intensity of the impact

The concept of the potential intensity of an impact is the acknowledgement at the outset of the project of the potential significance of the impact on the receiving environment. For example, SO₂ emissions have the potential to result in significant adverse human health effects, and this potential intensity must be accommodated within the significance rating. The importance of the potential intensity must be emphasised within the rating methodology to indicate that, for an adverse impact to human health, even a limited extent and duration will still yield a significant impact.



Within potential intensity, the concept of irreplaceable loss is considered. Irreplaceable loss may relate to losses of entire faunal or floral species at an extent greater than regional, or the permanent loss of significant environmental resources. Potential intensity provides a measure for comparing significance across different specialist assessments. This is possible by aligning specialist ratings with the potential intensity rating provided here. This allows for better integration of specialist studies into the environmental impact assessment. See Table 17 and Table 18 below.

Table 17: Criteria for impact rating of potential intensity of a negative impact

| Potential Intensity Descriptor | Definition of negative impact | Rating |
|--------------------------------|--|--------|
| High | Significant impact to human health linked to mortality/loss of a species/endemic habitat. | 16 |
| Moderate-High | Significant impact to faunal or floral populations/loss of livelihoods/individual economic loss. | 8 |
| Moderate | Reduction in environmental quality/loss of habitat/loss of heritage/loss of welfare amenity | 4 |
| Moderate-Low | Nuisance impact | 2 |
| Low | Negative change with no associated consequences. | 1 |

Table 18: Criteria for the impact rating of potential intensity of a positive impact

| Potential Intensity Descriptor | Definition of positive impact | Rating |
|--------------------------------|---|--------|
| Moderate-High | Net improvement in human welfare | 8 |
| Moderate | Improved environmental quality/improved individual livelihoods. | 4 |
| Moderate-Low | Economic development | 2 |
| Low | Positive change with no other consequences. | 1 |

It must be noted that there is no HIGH rating for positive impacts under potential intensity, as it must be understood that no positive spinoff of an activity can possibly raise a similar significance rating to a negative impact that affects human health or causes the irreplaceable loss of a species.

7.8 Likelihood of the impact

This is the likelihood of the impact potential intensity manifesting. This is not the likelihood of the activity occurring. If an impact is unlikely to manifest then the likelihood rating will reduce the overall significance. Table 14 provides the rating methodology for likelihood.

The rating for likelihood is provided in fractions in order to provide an indication of percentage probability, although it is noted that mathematical connotation cannot be implied to numbers utilised for ratings.

Table 19: Criteria for the rating of the likelihood of the impact occurring

| Likelihood Descriptor | Definition | Rating |
|-----------------------|--|--------|
| Improbable | The possibility of the impact occurring is negligible and only under exceptional circumstances. | 0.1 |
| Unlikely | The possibility of the impact occurring is low with a less than 10% chance of occurring. The impact has not occurred before. | 0.2 |
| Probable | The impact has a 10% to 40% chance of occurring. Only likely to happen once in every 3 years or more. | 0.5 |
| Highly Probable | It is most likely that the impact will occur and there is a 41% to 75% chance of occurrence. | 0.75 |



| Likelihood Descriptor | Definition | Rating |
|-----------------------|--|--------|
| Definite | More than a 75% chance of occurrence. The impact will occur regularly. | 1 |

7.9 Cumulative Impacts

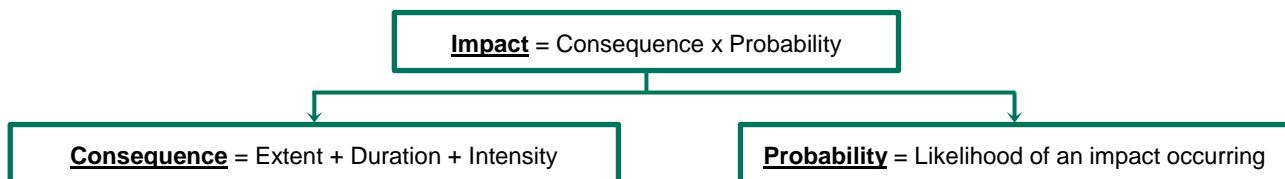
Cumulative impacts are reflected in the in the potential intensity of the rating system. In order to assess any impact on the environment, cumulative impacts must be considered in order to determine an accurate significance. Impacts cannot be assessed in isolation. An integrated approach requires that cumulative impacts be included in the assessment of individual impacts.

The nature of the impact should be described in such a way as to detail the potential cumulative impact of the activity.

7.10 Significance Assessment

The significance assessment assigns numbers to rate impacts in order to provide a more quantitative description of impacts for purposes of decision making. Significance is an expression of the risk of damage to the environment, should the proposed activity be authorised.

To allow for impacts to be described in a quantitative manner in addition to the qualitative description given above, a rating scale of between 1 and 5 was used for each of the assessment criteria. Thus the total value of the impact is described as the function of significance, which takes cognisance of extent, duration, potential intensity and likelihood. The significance rating process for impacts follows the established impact/risk assessment formula described in the flow diagram below.



$$\text{Impact Significance} = (\text{extent} + \text{duration} + \text{potential intensity}) \times \text{likelihood}$$

Table 20 provides the resulting significance rating of the impact as defined by the equation above.

Table 20: Significance rating formulas

| Score | Rating | Implications for Decision-making |
|---------|----------------|--|
| < 3 | Low | Project can be authorised with low risk of environmental degradation |
| 3 - 9 | Moderate | Project can be authorised but with conditions and routine inspections. Mitigation measures must be implemented. |
| 10 - 20 | High | Project can be authorised but with strict conditions and high levels of compliance and enforcement. Monitoring and mitigation are essential. |
| 21 - 26 | Fatally Flawed | Project cannot be authorised |

7.11 Surface Water Impact Rating

The impact rating for surface water in terms of water quality and water quantity at Medupi is described are listed in Table 21 to Table 23.



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Table 21: Water quality rating scale

| Activity | Nature of Impact | Impact Type | Extent | Duration | Potential Intensity | Likelihood | Rating | Mitigation | Interpretation |
|---|--|-------------|--------|----------|---------------------|------------|--|---|---|
| Water Quality (Pre-construction) | Pollution of natural surface water features. | Existing | 2 | 2 | 4 | 0.2 | 1.6 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit. There is therefore already an impact on the environment, however the SWMS appears to be well operated and maintained, therefore the existing impact is rated as low. Cumulatively there is not expected to be further impact to the environment because of where the activities are proposed to be located. With mitigation (SWMS as per GN704, eg. upgrading to include additional pollution controls dams in a phased manner) the residual surface water pollution impact will be kept low due to the probability of dirty water spilling over into the environment from Medupi Power Station. Proper maintenance of the SWMP will reduce the rating to low. Ongoing surface water monitoring is important to ensure that this trend continues, especially during high rainfall events. | Because of the existing facility with SWMS in place and that appears to be well operated and maintained, rated as low impact. |
| | | Cumulative | 2 | 2 | 4 | 0.2 | 1.6 – LOW | | Because of the existing facility with SWMS in place and that appears to be well operated and maintained, rated as low impact. |
| | | Residual | 2 | 2 | 4 | 0.2 | 1.6 – LOW | | Because of the existing facility with SWMS in place and that appears to be well operated and maintained, rated as low impact. |
| Water Quality (Construction) | | Existing | 2 | 3 | 4 | 0.5 | 4.5 – MOD | During construction it is possible that there may be increased contaminants reaching the surface water resources due to alterations that need to be made to the SWMS. These impacts should be reduced once construction is complete. With mitigation during construction (Existing SWMS maintained and well operated to deal with an increased pollutant load as per GN704), the residual surface water pollution impact will be reduced. | Because of the existing facility Surface water quality is already rated as moderate impact. |
| | | Cumulative | 2 | 3 | 4 | 0.5 | 4.5 – MOD | | Water quality may be slightly further impacted but will remain a moderate impact with all cumulative impacts. |
| | | Residual | 2 | 2 | 4 | 0.2 | 1.2 – LOW/ MOD | | Water quality will reduce to low impact with mitigation. |
| Water Quality (Operational) | | Existing | 2 | 2 | 4 | 0.2 | 1.6 – LOW | A SWMS is in place and so the existing impacts should be low. The grab sample taken does appear to indicate this and the SWMS on site appears to be well operated and maintained. During operation a well operated and maintained SWMS with addition PCDs and channels are required and phased in over time, where clean and dirty water is separated according to GN704, channels are kept clean and PCDs do not overflow, will ensure limited surface water pollution. | Low impact if the SWMS is designed, operated and maintained according to GN704 |
| | | Cumulative | 2 | 2 | 4 | 0.2 | 1.6 – LOW | | Low impact if the SWMS is designed, operated and maintained according to GN704 |
| | | Residual | 2 | 2 | 4 | 0.2 | 1.6 – LOW | | Low impact if the SWMS is designed, operated and maintained according to GN704 |
| Water Quality (Decommissioning) | Existing | 2 | 3 | 4 | 0.5 | 4.5 – MOD | As with construction, decommissioning will have an increased load of pollutants where infrastructure is removed, however this should be short term and if adequate storm water management measures are put in place, then this should be limited to the site so the impact would be moderate. Post decommissioning the impact should once again be reduced to low as long as the area is well rehabilitated where infrastructure is removed and the SWMP around those facilities that will stay in place are upgraded as necessary and are maintained. | A moderate impact could be expected once decommissioning of the site occurs | |
| | Cumulative | 2 | 3 | 4 | 0.5 | 4.5 – MOD | | A moderate impact could be expected once decommissioning of the site occurs | |
| | Residual | 2 | 2 | 4 | 0.2 | 1.6 – LOW | | Reduce to low impact if the SWMS is upgraded as necessary, and maintained, and areas where infrastructure is removed are adequately rehabilitated according to a rehabilitation plan. | |



Table 22: Water quantity rating scale (surface runoff reduction)

| Activity | Nature of Impact | Impact Type | Extent | Duration | Potential Intensity | Likelihood | Rating | Mitigation | Interpretation |
|-------------------------------------|--|-------------|--------|----------|---------------------|------------|---|--|--------------------------------|
| Runoff Reduction (Pre-construction) | Reduction of the surface water runoff footprint. | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit. There is therefore already an impact in respect of reducing the flow to the Sandloop. However the SWMS appears to be well operated and maintained so that the clean water around the site reaches the river. The existing impact is therefore rated as low. Cumulatively because the new facilities will be part of the existing footprint there is no further impact in respect of run-off reduction. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| Runoff Reduction (Construction) | | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit, so no further impact in respect of run-off reduction is expected. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| Runoff Reduction (Operational) | | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit, so no further impact in respect of run-off reduction is expected. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| Runoff Reduction (Decommissioning) | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The run-off may increase as areas are rehabilitated, so this should be a limited but positive impact. | Runoff reduction is low impact | |
| | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact | |
| | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact | |

Table 23: Water quantity rating scale (Flooding)

| Activity | Nature of Impact | Impact Type | Extent | Duration | Potential Intensity | Likelihood | Rating | Mitigation | Interpretation |
|-----------------------------|---------------------------------|-------------|--------|----------|---------------------|------------|-----------|---|--------------------------------|
| Flooding (Pre-construction) | Flooding of nearby watercourses | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit. In respect of potential flooding, the existing SWMS appears to be adequately designed to cater for the existing facilities. The runoff around the facility in the clean areas is not markedly changed for the sub-catchment of the Sandloop. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| Flooding (Construction) | | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit. In respect of potential flooding, the existing SWMS appears to be adequately designed to cater for the existing facilities. The runoff around the facility in the clean areas is not markedly changed for the sub-catchment of the Sandloop. It will be important to do the relevant upgrades in the phased approach proposed, and then to operate and maintain the system optimally. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |



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| Activity | Nature of Impact | Impact Type | Extent | Duration | Potential Intensity | Likelihood | Rating | Mitigation | Interpretation |
|----------------------------|------------------|-------------|--------|----------|---------------------|------------|-----------|--|--------------------------------|
| Flooding (Operational) | | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The Medupi Power Station already has a footprint into which the proposed activities will fit. In respect of potential flooding, the existing SWMS appears to be adequately designed to cater for the existing facilities. The runoff around the facility in the clean areas is not markedly changed for the sub-catchment of the Sandloop. The SWMS will need to be optimally operated and maintained. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| Flooding (Decommissioning) | | Existing | 1 | 1 | 1 | 0.1 | 0.3 – LOW | The run-off may increase as areas are rehabilitated, so this should be a limited, but positive impact. | Runoff reduction is low impact |
| | | Cumulative | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |
| | | Residual | 1 | 1 | 1 | 0.1 | 0.3 – LOW | | Runoff reduction is low impact |



8.0 SPECIALIST OPINION ON SLUDGE & SALTS TRUCKING IMPACT

This section provides specialist opinion on the significance of the surface water impacts for the proposed trucking of sludge and salts from Medupi Power Stations' proposed temporary hazardous waste storage area to an appropriately licensed existing hazardous waste facility outside of the Medupi Power Station study area. Detailed description of the production, processing and disposal of sludge and salts at Medupi can be found in the Medupi Final Scoping Report on DEA REF: 14/12/16/3/3/3110 of June 2015 by Zitholele Consulting.

This section aims to describe the potential surface water impacts that could arise from the disposal of sludge and salts from Medupi to an existing licensed hazardous disposal facility outside of the study area. In general, the FGD retrofit activities, other than the salts and sludge disposal, will occur within the Medupi Power Station footprint and at the existing licensed disposal facility. The surface water resources along the path of transportation of sludge and salts within the study area are in question. Trucking of salts and sludge from Medupi can be summarised as follows:

- The nature of materials being transported, the mode of transportation, the route chosen for transportation, and the distance over which the materials are transported were of most significance in assessing the potential surface water impacts;
- The assumption at this stage is made that the hazardous waste disposal facility is within a 15km radius of Medupi Power Station, the mode of transportation is trucks, and the transportation route does intersect with surface water resources;
- Based on the above considerations and assumptions, it can be highlighted that the trucking of salts and sludge from Medupi to the licensed hazardous waste site will pose a medium potential risk impact to the water resources in the study area;
- The medium, rather than high, risk impact assessment rating is in light of the fact that Medupi Power Station has taken significant steps in investigating this matter beforehand. Various specialist studies have been commissioned to investigate this matter and its associated risks thoroughly and give specialist opinions as well as mitigation measures where possible; and
- It is therefore in our opinion that the transportation of salts and sludge from Medupi Power Station to an appropriately licensed existing hazardous waste facility outside of the study area will not pose a serious threat to water resources in the region.

9.0 MITIGATION AND MANAGEMENT MEASURES

Based on the potential surface water impacts identified in 7.0, the following section describes the associated mitigation measures that Eskom is required to implement at Medupi Power Station, aimed at reducing potential negative surface water impacts and enhancing potential positive environmental and social impacts.

Table 24 and Table 25 present mitigation proposed for the construction and operational phases of the project to limit surface water impacts and get a good understanding of the potential load of contaminants that would report to the Mokolo via the Sandloop.

Floodline

The footprint of the proposed Ash Disposal Facility is 925.86 ha (9.26 km²). The following summary can be made from the floodline study:

- The 1:100 year floodline encroaches on the ADF footprint;
- The south-western portion of the proposed ADF footprint will be mostly affected by the 1 in 100 flood;
- The ADF project disturbance boundary is located within the Probable Maximum Flood (PMF); therefore



- To avoid flooding and contamination of the downstream environment through the transportation of pollutants from the ADF, the existing footprint should be re-designed or decreased in size.
- Water quality monitoring in the small dam on the south west corner must be undertaken monthly or in accordance with the relevant water use authorisation.

Loss of Catchment Flows

- The existing Medupi site and ADF site have a combined area of approximately 1,874 ha (18.7km²) which equates to 1.03% of quaternary catchment A42J with a catchment area of 1,812km² (WRC, 2012);
- The Sandloop River tributary has an estimated catchment area of 4,467 ha (44.7km²). The reduction in catchment area from the Medupi site and ADF site of approximately 1,874 ha (18.7km²) equates to a 49.95% decrease in catchment area; and
- It is therefore anticipated that during the operational phase of the ADF, there will be a reduction in the total runoff reporting to the Sandloop River tributary, however limited reduction to the Mokolo system.



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Table 24: Proposed surface water mitigation measures - construction phase

| Activity | Impact | Industrial Process | Proposed Mitigation |
|---|---|-----------------------|--|
| Site clearing for construction of FGD and associated waste disposal areas | <ul style="list-style-type: none"> ■ Removal of topsoil leading to erosion and increased sedimentation in the surface water resources; ■ Operation of equipment may lead to spillage of oil that may find its way to the surface water resources; ■ Polluted surface water resources have reduced availability for downstream water users. | FGD | <p>As this will be within the existing footprint, it is unlikely that there will be considerable impacts from the removal of vegetation and/or topsoil during excavation. However, this aspect should be considered and managed to reduce erosion which could cause siltation of the surrounding surface water resources.</p> <p>Removal of topsoil should be done systematically, only clearing the necessary areas at a time. As possible, clean and dirty surface water channels should be constructed to divert runoff separately to the appropriate storage dams (dirty water to the PCD to avoid eroded soils entering the clean water areas).</p> |
| Construction activities | <ul style="list-style-type: none"> ■ Operation of equipment may lead to spillage of oil that may find its way to the surface water resources; ■ Chemical contaminants from building material may enter the water resources ■ Polluted surface water resources have reduced availability for downstream water users | ADF Sludge & Salts | <p>As this will be within the existing footprint, it is unlikely that there will be considerable impacts from the removal of vegetation and/or topsoil during excavation. However, this aspect should be considered and managed to reduce erosion which could cause siltation of the surrounding surface water resources.</p> <ul style="list-style-type: none"> ■ Removal of topsoil should be done systematically, only clearing the necessary areas at a time. As possible, clean and dirty surface water channels should be constructed to divert runoff separately to the appropriate storage dams (dirty water to the PCD to avoid eroded soils entering the clean water areas). ■ The existing SWMS will need to be optimally operated and maintained. ■ Ongoing monitoring of the surface water for: <ul style="list-style-type: none"> ■ pH, Total Dissolved Solids, Electrical Conductivity, Alkalinity, potassium, calcium, sodium, chloride, fluoride, sulphate, nitrate, ammonium, Total Hardness, Metals: arsenic, beryllium, cadmium, barium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, uranium, vanadium and zinc using ICP-MS), |



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| Activity | Impact | Industrial Process | Proposed Mitigation |
|----------|--------|--------------------|--|
| | | | orthophosphate, Total Suspended Solids, Oil and Grease <ul style="list-style-type: none"> ▪ Monthly when water is available or after a rain event |

Table 25: Proposed surface water mitigation measures - operational phase

| Activity | Impact | Industrial Process | Proposed Mitigation |
|--|---|------------------------|--|
| Operation of the FGD system within the Medupi Power Station Footprint | Increased contaminants in the area from machinery and operation of the plant | FGD | <ul style="list-style-type: none"> ■ Upgrading of the existing SWMS to comply to GN704; ■ Optimal operation and maintenance of the SWMS to ensure PCDs do not overflow; sediment and any other obstructive material is regularly removed from dams and channels; |
| Operation of the railway yard and limestone and gypsum handling facilities between the Medupi Power Station and existing ADF | Increased contaminants in the area from machinery and disposal | Sludge & Salts | <ul style="list-style-type: none"> ■ Optimal operation and maintenance of the SWMS to ensure PCDs do not overflow; sediment and any other obstructive material is regularly removed from dams and channels; |
| Disposal of ash and gypsum together on the existing ADF | Potential for surface water contamination by trace elements associated with the ash and gypsum. | ADF; Sludge & Salts | <ul style="list-style-type: none"> ■ As the south west corner of the footprint is in the 1:100 floodline, measure on how the existing infrastructure can be managed to prevent spills to the river. Water quality monitoring of the small dam in the South west corner needs to be undertaken monthly. ■ Classification of the gypsum purity to assess alternative disposal options; ■ Ongoing monitoring of the surface water at the points identified for: <ul style="list-style-type: none"> ▪ pH, Total Dissolved Solids, Electrical Conductivity, Alkalinity, potassium, calcium, sodium, chloride, fluoride, sulphate, nitrate, ammonium, Total Hardness, Metals: arsenic, beryllium, cadmium, barium, chromium, copper, lead, mercury, molybdenum, nickel, selenium, uranium, vanadium and zinc using ICP-MS), |



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| Activity | Impact | Industrial Process | Proposed Mitigation |
|----------|--------|--------------------|--|
| | | | orthophosphate, Total Suspended Solids, Oil and Grease <ul style="list-style-type: none">▪ Monthly when water is available or after a rain event |



10.0 CONCLUSIONS AND RECOMMENDATIONS

The following conclusions were drawn and recommendations made from the Medupi surface water impact assessment study:

- The footprint of the proposed Ash Disposal Facility is 925.86 ha (9.26 km²). The following summary can be made from the floodline study:
 - The 1:100 year floodline encroaches on the ADF footprint;
 - The south-western portion of the proposed ADF footprint will be mostly affected by the 1 in 100 flood;
 - The ADF project disturbance boundary is located within the Probable Maximum Flood (PMF); therefore
 - To avoid flooding and contamination of the downstream environment through the transportation of pollutants from the ADF, the existing footprint should be re-designed or decreased in size.
- The existing Medupi site and ADF site have a combined area of approximately 1,874 ha (18.7 km²) which equates to 1.03% of quaternary catchment A42J with a catchment area of 1,812 km² (WRC, 2012);
- The Sandloop River tributary has an estimated catchment area of 4,467 ha (44.7 km²). The reduction in catchment area from the Medupi site and ADF site of approximately 1,874 ha (18.7 km²) equates to a 49.95% decrease in catchment area; and
- It is therefore anticipated that during the operational phase of the ADF, there will be a reduction in the total runoff reporting to the Sandloop River tributary, however limited reduction to the Mokolo system.
- Natural on land surface water drainages are absent in the existing footprint of Medupi Power Station and will therefore not be impacted by the Flue Gas Desulphurisation (FGD) Retrofit project.
- The 100-year floodline of the Sandloop River in the area of the ADF encroaches on the ADF footprint in the south western corner and this may have a detrimental effect in the event of a major flood event. Should the ADF operate within the 1:100 year floodline, the risk of pollutant transportation towards downstream water users during a flood event will be elevated. This will include flooding of the disposal facility and entrainment of waste materials and sediments downstream, making the management of the facility during significant storm events very difficult.
- If sound engineering flood control and prevention measures are not put in place, the contents of the ADF are likely to be washed away into the receiving environment in the event of a 1:100 flood. Statistically, the 1:100 year flood event refers to the mathematical probability of this flood magnitude occurring once over a 100-year period. However, in reality this flood magnitude may occur more than once in 100 years. With this in mind, the 20-year lifespan of the ash disposal facility should not be directly compared to the 1:100 flood event. ADF design and flood mitigation measures should be based on the 1:100 year flood event.
- Storm water that is generated within the Medupi Power Station, including the ADF, as a result of rainfall is a route by which pollutants may be mobilised and transported into the receiving downstream environment. The National Water Act (NWA) prohibits the discharge of any effluent (including contaminated storm water) into any water resources.
- To prevent possible pollution of the receiving surface water environment, dirty water containment structures should be designed, constructed, maintained and operated such that they do not spill over more than once in 50 years. A minimum freeboard of 0.8 m above full supply level (FSL) must also be maintained as per GN704 requirements (flow-based hydraulic sizing requirements). Water accumulated in the containment facility during the wet season should be used as a priority in the process water circuit to ensure that the capacity requirements are not compromised during periods of heavy and/or extended rainfall.



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- It is recommended that an update to both the storm water management plan (SWMP) and the existing water balance be undertaken such that it caters for the proposed FGD and ADF infrastructure as well as be designed and operated in line with the DWS's GN704.
- During construction and times of major disturbances to land cover, it is recommended that sound engineering measures are put in place to protect the receiving surface water environment. It is also recommended that, where possible, construction and land cover disturbance is carried out during the dry season to avoid the washing away of materials by surface runoff (post-construction sediment and erosion control).
- If possible, it is recommended that a detention (dry) pond be constructed at or near the discharge point of the clean water drainage system before it enters the environment. This pond will be constructed for the purpose of flood control as well as storm water runoff treatment. This pond will function to settle suspended sediments and other solids typically present in storm water runoff. In the event of a major storm, the detention pond will slow down water flow and hold it for a short period of time before releasing it to the environment.
- It is strongly recommended that the proposed water quality monitoring programme be strictly followed and sustained so that chemical constituent levels can be monitored and analysed over time. Pollution of surrounding surface water features should be avoided at all costs during the lifespan of the Medupi Power Station project. In the unfortunate occurrence of surface water resources pollution, swift and effective corrective measures should be implemented and the relevant authorities notified without delay.
- With respect to the transportation of sludge and salts from Medupi to a hazardous waste disposal site, it is recommended that a route selection study be carried out to determine the least potential water surface impacts, considering other factors such as the traffic impact assessment. From a surface water perspective, a route via a national road (highway) would be most appropriate as the likelihood of accidents and spillages due to poor road conditions will be minimised.

The impact assessment showed that most impacts were low after mitigation. If the impacts are properly mitigated and Best Management Practices followed at all times, the identified potential impacts can be reduced to negligible.

GOLDER ASSOCIATES AFRICA (PTY) LTD.

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APPENDIX A

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APPENDIX B

**General layout of the existing ADF and storm water
management philosophy**

As a global, employee-owned organisation with over 50 years of experience, Golder Associates is driven by our purpose to engineer earth's development while preserving earth's integrity. We deliver solutions that help our clients achieve their sustainable development goals by providing a wide range of independent consulting, design and construction services in our specialist areas of earth, environment and energy.

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|---------------|-------------------|
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